

FCR (Park Lawn) LP and CPPIB
Park Lawn Canada Inc.

2150 Lake Shore

Preliminary Geotechnical Study

Issue 01 | May 15, 2020

This report takes into account the particular instructions and requirements of our client.

It is not intended for and should not be relied upon by any third party and no responsibility is undertaken to any third party.


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Executive Summary

This geotechnical study has been prepared in accordance with the requirements for a Geotechnical Study set out by the City of Toronto¹.

The current Master Plan features a range of land uses including a new public park, and a diverse mix of residential, retail, service, entertainment and employment uses and a range of building types. Fifteen towers are proposed on the site with heights ranging from 16 to 70 storeys. The current Master Plan includes 6 phases, each of which includes a basement ranging from 3 to 5 levels to a minimum basement slab elevation of +65.5 masl (meters above sea level, approximately 20 mbgs – meters below ground surface).

This report summarizes the findings from available geotechnical investigation and provides assessment on the soil, bedrock and groundwater characteristics of the subject property to determine its feasibility and stability to accommodate the proposed development, which includes preliminary design and construction recommendations for site preparation, foundations, floor slabs, retaining walls, temporary shoring system, underground services, pavement structure, earthquake consideration and dewatering. In addition, this report provides discussion on potential risks, mitigation measures and monitoring programs for the proposed development.

This report is based on previous ground investigation work carried out at the site and a review of historical records within and surrounding the site. Although ground investigations have been carried out within the site, ground investigation has yet to be completed specifically for the current proposed development. As a result, analysis in this report is based on available field and lab testing and does not include data obtained from slug tests, pumping tests, or long-term groundwater monitoring. Gaps in the available data for further design stages will be addressed by further site investigation and analysis. The results of these further investigations can be provided to the City of Toronto if required.

¹ Refer to the following City of Toronto website for more details <<https://www.toronto.ca/wp-content/uploads/2018/08/97bb-Hydrological-Review-August-2018.pdf>>.

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1 Introduction

1.1 Project Description

In October 2019, FCR (Park Lawn) LP and CPPIB Park Lawn Canada Inc. (“the Owners” or “FCR”) made an application for an Official Plan Amendment (OPA) in support of a proposed Master Plan for the redevelopment of the 27.7 acre / 11.2 hectare site located on the northeast corner of Park Lawn Road and Lake Shore Boulevard West, municipally known as 2150-2194 Lake Shore Boulevard West and 23 Park Lawn Road site (“the site” or “2150 Lake Shore”), as shown in Figure 1.

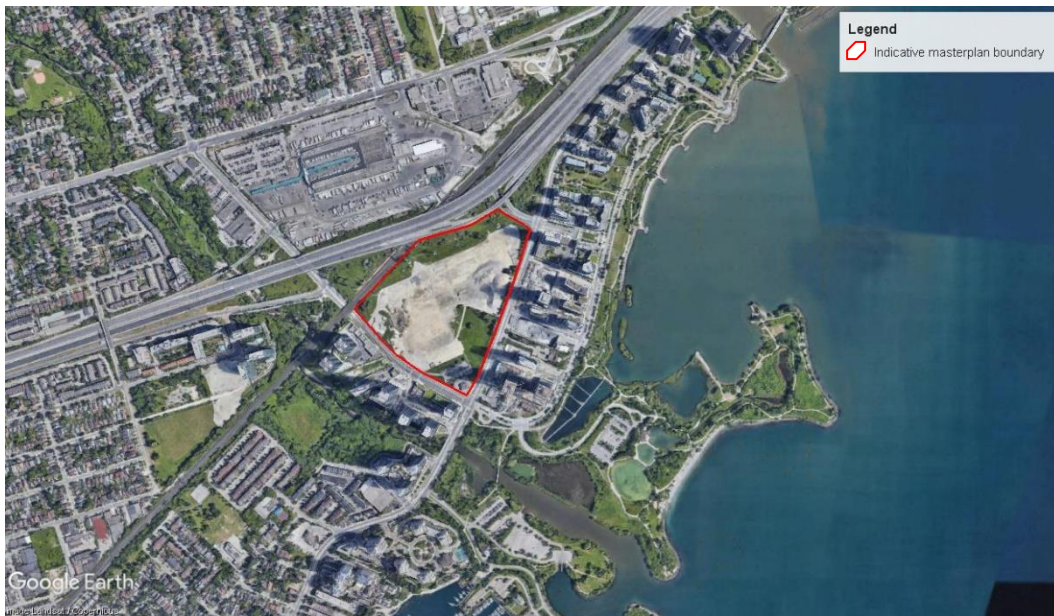


Figure 1: Site Location and Boundary

The original Master Plan proposal envisioned a vibrant, mixed-use, transit-oriented redevelopment of the site. The Master Plan included a new Park Lawn GO Station, related TTC transit improvements, a fine-grained network of new streets and connections, a range of new open spaces including a new public park, and a diverse mix of residential, retail, service, entertainment and employment uses.

The current Master Plan features the same variety of land uses with a range of building types that blend forms and uses, and respond to the distinct geometry of the proposed street and block pattern. Fifteen towers are proposed on the site with heights ranging from 16 to 70 storeys, with the tallest towers generally clustered near the GO Station. The towers feature generous separation distances and are interspersed with a range of standalone mid-rise and low-rise building typologies to create a sense of place and urban fabric that appears to have evolved over time.

1.2 Proposed Development

The proposed site contains several mix-use buildings (residential, employment and retail), two schools, a 1 ha park, several open spaces, a new train station on the Lake Shore GO line, a TTC streetcar loop, and a series of public and private roadways, as shown in Figure 2.



Figure 2: Proposed Development

1.3 Scope and Limitation

Arup was retained by FCR (Park Lawn) LP and CPPIB Park Lawn Canada Inc. to prepare a Geotechnical Study for the Zoning Bylaw Amendment application.

This report summarizes the findings from available geotechnical investigation and provides assessment on the soil, bedrock and groundwater characteristics of the subject property to determine its feasibility and stability to accommodate the proposed development, which includes preliminary design and construction recommendations for site preparation, foundations, floor slabs, retaining walls, temporary shoring system, underground services, pavement structure, earthquake consideration and dewatering. In addition, this report provides discussion on potential risks, mitigation measures and monitoring programs for the proposed development.

This report takes into account the particular instructions and requirements of our client. It is not intended for and should not be relied upon by any third party and no responsibility is undertaken to any third party.

Arup's responsibility is limited to documenting the information encountered at the borehole locations, at the time of their determination during preparation of this report. Any discrepancies between this report and the borehole logs, information on the borehole logs shall prevail. Ground and groundwater information may vary between and beyond boreholes.

2 Site Condition

The site is located at 2150 Lake Shore Boulevard West, in the Etobicoke-Lakeshore area of the Toronto City District of Etobicoke-York. The site is approximately 11.2 hectares and broadly triangular in shape. Bounded to the northwest by CN railway line and the Gardener Expressway eastbound off ramp, bounded to the east by Lake Shore Boulevard West, and the southwest by Park Lawn Road.

2.1 Topography

The site is generally flat, with existing elevation across the site typically ranging from approximately +84 masl and +86 masl. Beyond the typical ranges, the site elevation increases several metres at the northern boundary due to fill slopes associated with the adjacent the Canada National Railway and the Gardener Expressway east bound off ramp. Beyond the typical ranges, the site elevation reduces in the southern corner. It should be noted that significant grading to form the final formation level for the proposed development.

2.2 Existing Geotechnical Features

Existing geotechnical features include a fill slope with a 2:1 to 2.5:1 gradient, measuring approximately 10 m in height and 250 m in length forming the southern slope of the embankment upon which the Gardener Expressway east bound exist ramp is located, forming the north-northwestern boundary of the site; a retaining wall measuring up to approximately 5 m in height and 100 m in length, alongside an onsite access road, and in part, in close proximity to Canadian National Railway line passing along the northwestern boundary of the site. A small slope measuring up to approximately 2 m in height and 65 m in length along the northern boundary of the site. These features allow differing ground elevation within the site and surrounding area and will need to be further assessed as part of the site development.

3 Land Use History

3.1 Historical Maps and Aerial Photographs

A review of available historical maps and aerial photographs for the area has been conducted to determine the site land use history, which is summarized in Table 1, below.

Table 1: Summary of historical Maps and Aerial Photographs

Year	Sheet/Photograph No.	Details
1903	Plan of the city of Toronto.	The site is bounded by Lake Shore Road to the south, Salisbury Road (currently Park Lane Road) to the southwest, and CN Railway to the north. No development is shown within site.
1908	City of Toronto Contour Map	The site is bounded by Lake Shore Road to the south, Salisbury Road (currently Park Lane Road) to the southwest, and CN Railway to the north. No development is shown within site.
1921	Toronto Transportation Commission – Contour Map of Toronto District	The site is shown to be bounded by Lake Shore Road to the south, Canadian Pacific Railway to the north. No development is shown within site. Site elevation is shown to be approximately +84 masl.
1932	Province of Ontario Department of Mines. Map No. 41g. The Pleistocene of the Toronto Region	The site is shown to be bounded by Lake Shore Road to the south, Canadian Pacific Railway to the north, and Park Lane Road to the southwest. The site is labelled as “Brick Yards”.
1937	City of Toronto Planning Board	The site is bounded by Lake Shore Road, Park Lane Road, CN Rail, and Queen Elizabeth Way (now the Gardener Expressway). No development is shown within site.
1947	Aerial Photograph 21C	The site is bounded by Lake Shore Road, Park Lane Road, CN Rail, and Queen Elizabeth Way (now the Gardener Expressway). The site appears to have been largely cleared of vegetation but remains undeveloped with the exception of several small structures towards the west of the site. To the south of the site, Humber Bay Park East and West are not yet reclaimed and remain as open water.
1950	Aerial Photograph 21C	The northern portion of the Mr. Christie’s Bakery building is present on the site. The water tower in the northern portion of the site has also been constructed.

Year	Sheet/Photograph No.	Details
1956	Aerial Photographs 183 and 184	The southern portion of the building that is currently occupied by Mr. Christie's Bakery is present on the site, completing the building in its current state. The building that is currently the BMO bank is present in the southern corner of the site.
1959	Aerial Photograph 11	No significant change within the site. Reclamation to the south of the site, forming the land for what is now Marine Parade Drive and adjacent developments.
1960 to 1971	Aerial Photographs	No significant change within the site. Continued reclamation to the south.
1973	Aerial Photograph 35	No significant change within the site. Ongoing reclamation to the south of the site, to what is now partially Humber Bay Park East and West
1975	Aerial Photograph 29	Construction of the Gardener Expressway eastbound off ramp bounding the north of the site has been completed.
1976 to 1992	Aerial Photographs	No significant change within the site.
2002 to 2017	Google Earth	No significant change within the site.
2018	Google Earth	The former Mr. Christie's Bakery has been removed from the site. The BMO building in the southern corner of the site remains.

3.2 Published Historical Information

The following information is credited to the Etobicoke Historical Society, who have compiled historical information on Humber Bay and other neighbourhoods throughout Etobicoke.

Lake Shore Boulevard West (called Lake Shore Road until 1959), forms the southeast border of the site and was first surveyed in 1791 along the path of an ancient aboriginal trail, making it one of the oldest roads in Ontario. Much of the local area, including 2150 Lake Shore Boulevard has historically been used as farmland. In 1853 the railway, currently the Canadian National Railway was constructed which now forms part of the northern boundary of the site. In 1940 Queen Elizabeth Way (now the gardener Expressway) was constructed. Within the site there have been five brickyards operating, the largest until the 1930s, making bricks from local sandy clay. Other early industries included Carson Cement Block and Humber Coal and Supply. In 1948, Christie Brown & Co. built a large bakery on the site, which closed in 2014.

4 Review of Existing Information

4.1 Published Geological Information

The map of Quaternary Geology of Toronto and the Surrounding Area (1980) indicates that the site is underlain by Older Lake Deposits of silt and clay, as shown in Figure 3. Beneath the Older Lake Deposits there is potential for presence of the Older Glacial Till consisting of silty clay to silt and clayey sand. Beneath the Quaternary Deposits the bedrock is formed of shale, interbedded with siltstone and occasional limestone.

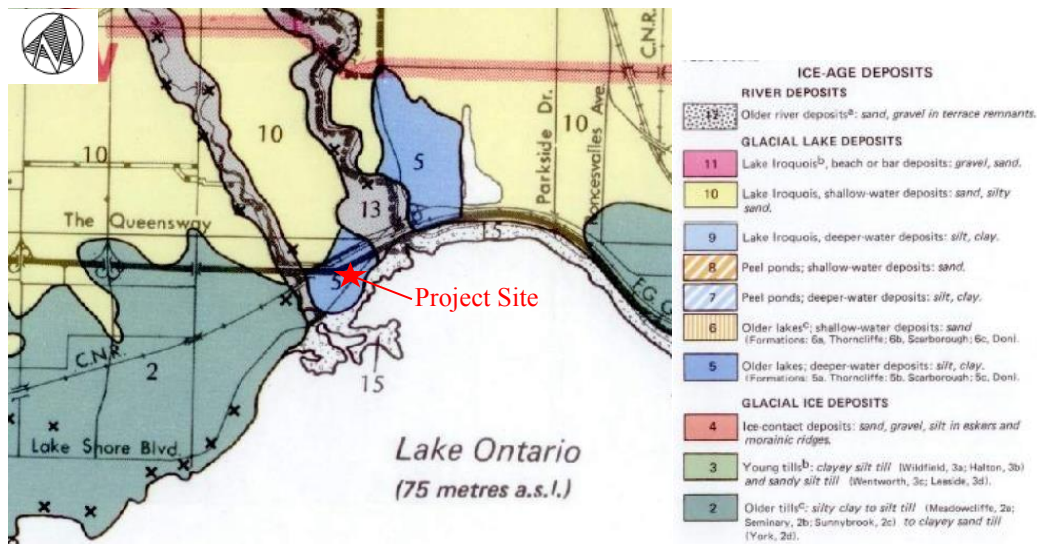


Figure 3: Regional Geology of Project Site (Quaternary Geology of Toronto and the Surrounding Area (1980))

4.2 Geotechnical Reports and Information

4.2.1 Historical Geotechnical Reports and Boreholes

A search of ground investigation information within the site and surrounding area has been carried out via the following resources:

- Ontario Ministry of Transport Foundation Library;
- Ontario Geotechnical Boreholes. Maintained by Ontario Geological Survey (OGS) and the Ministry of Natural Resources (MNR); and
- Toronto Development Projects Planning Applications.

Table 2, below, summarizes reports and eighteen (18) boreholes that have been considered in preliminary determination of the site-specific ground conditions.

Table 2: Summary of Available Geotechnical Reports

Source	Document Type	Relevant Boreholes	Borehole Locations
Ontario Ministry of Transport Foundation Library	Foundation Investigation Report – 30M11-094 (1970)	BH9 and BH10	Boreholes located at northern boundary of site, adjacent to Gardiner Expressway east bound off ramp
	Foundation Investigation Report – 30M11-097 (1970)	BH107 and BH109	Boreholes located at northeastern boundary of site.
Ontario Geotechnical Boreholes (Maintained by OGS and MNR)	Summary of borehole details including completion date, strata summary, and ground water depth.	604058 604070 604069 655256 655257 604066 604068 604067 604054 604053	Within the 2150 Lake Shore Boulevard site, in close proximity to the northwestern boundary.
Toronto Development Projects Planning Applications	Preliminary Geotechnical Investigation for Proposed High-rise Buildings 2161 to 2165 Lake Shore Boulevard West.	BH13-1 BH13-2 BH13-3 BH13-4	South of Lake Shore Boulevard West.

4.2.2 Site Specific Geotechnical Reports

Two site specific geotechnical reports are summarized in Table 3. Thirty-eight borehole logs and one test pit log were provided in the site specific geotechnical reports. Eighteen (18) out of the thirty-eight (38) boreholes and one (1) test pit were carried out for environmental sampling or well installation with no geotechnical in situ tests or laboratory tests having been carried out for these boreholes.

Table 3: Summary of Project Specific Geotechnical Reports

Date	Source	Document Type	Relevant Boreholes and Test Pits
February 2013	Conestoga-Rovers & Associates	Preliminary Geotechnical Investigation, 2150 Lake Shore Boulevard West Toronto, Ontario	MW1-13, MW2-13, MW3-13, MW4-13, MW5-13, MW6-13, MW7-13, MW8-13, MW9-13, *TP1-03, *BH2-04, *BH3-04, *BH4-04, *BH5-04, *BH6-04, *BH7-

Date	Source	Document Type	Relevant Boreholes and Test Pits
			04, *BH8-04, *BH9-04, *BH10-04, *BH11-04, BH1, BH2, MW1-04, MW2-04, MW3-04, *MW5-04, *MW6-04, *BH201-05, *BH202-05, *BH203-05, *BH204-05 and *BH205-05
January 2015	Golder Associates	Preliminary Geotechnical Investigation, 2150 Lake Shore Boulevard West Toronto, Ontario	15-1, 15-2, 15-3, 15-4, 15-5, *MW14-4, *MW14-6
* Boreholes or test pits were carried for environmental sampling or well installation with no geotechnical laboratory or in situ tests.			

5 Ground and Groundwater Conditions

5.1 General

Based on the project specific geotechnical reports, the subject property is underlain by the overburden materials comprising a layer of earth fill overlaying the native silty clay to silt deposits, and the native silty clay to silt deposits are underlain by shale bedrock of Georgian Bay Formation.

The above subsurface condition is consistent with the general subsurface conditions interpreted from the historical geotechnical reports from adjacent sites.

5.2 Ground Conditions

The following soil stratigraphy is interpreted based on the site specific geotechnical boreholes.

5.2.1 Topsoil, Concrete and Asphalt

Based on the site specific geotechnical reports, topsoil was encountered in seven (7) boreholes drilled in the landscaped area; concrete was encountered in five (5) boreholes drilled within the demolished building, and asphalt was encountered in twenty (20) boreholes and one (1) test pit carried out in the demolished parking lot. Details of these surface cover are summarized in Table 4.

Table 4: Topsoil, Concrete and Asphalt Thickness and Location Summary

Description	Min. Thickness (mm)	Max. Thickness (mm)	Borehole Encountered
Topsoil	30	150	MW1-13, MW3-13, MW5-13, MW6-13, MW7-13, BH1 and BH2
Concrete	100	200	BH11-04, BH201-05, BH202-05, BH203-05, BH204-05 and BH205-05
Asphalt	60	210	BH15-1, BH15-2, BH15-3, BH15-4, BH15-5, MW2-13, MW4-13, MW8-13, MW9-13, BH2-04, BH3-04, BH4-04, BH5-04, BH8-04, BH9-04, BH10-04, MW1-04, MW2-04, MW3-04, MW5-04, MW6-04 and TP1-03

5.2.2 Earth Fill

Granular fill, ranging from 50 mm to 1.27 m in thickness, was encountered beneath the pavement structure or concrete slab in twenty (20) boreholes summarized in Table 5. The granular fill generally comprises brown or grey sand and gravel with

trace to some silt. Based on the SPT-N values and moisture content summarized in Table 6, the granular fill is moist to wet and is in very loose to compact condition.

Earth fill, ranging from 0.3 m to 3.96 m in thickness, was recorded in twenty (20) boreholes summarized in Table 5. The earth fill comprises sand, silt and silty clay materials. Based on the SPT-N values and moisture content summarized in Table 6, the earth fill material is moist to wet and in very loose to dense or very soft to very stiff condition. The high moisture content values indicated the presence of organic matters.

Table 5: Earth Fill Thickness and Location Summary

Description	Min. Thickness (mm)	Max. Thickness (mm)	Borehole Encountered
Granular Fill	50	1270	BH15-1, BH15-2, BH15-3, BH15-4, BH15-5, MW4-13, MW8-13, BH2-04, BH3-04, BH4-04, BH5-04, BH6-04, BH7-04, BH9-04, BH10-04, MW5-04, MW6-04, BH201-05, BH202-05 and BH203-05.
Earth Fill	300	3960	BH15-1, BH15-4, MW1-13, MW3-13, MW6-13, MW7-13, MW9-13, BH1, BH2, TP1-03, BH6-04, BH7-04, BH8-04,

			BH10-04, BH11-04, MW1-04, MW2-04, MW3-04, MW-5-04 and MW6-04
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Table 6: SPT-N and Moisture Content Summary for Earth Fill

Description	Min. Moisture Content	Max. Moisture Content	Min. SPT-N Value	Max. SPT-N Value
Granular Fill	4	16	4	27
Earth Fill	7	42	0	34

5.2.3 Silty Clay and Clayey Silt

Below the topsoil, concrete, asphalt pavement and earth fill, natural deposits of clayey silt to silty clay were encountered in majority of the boreholes and extended to the bedrock at depths ranging from 4.1 m to 7.6 m below ground surface in eleven (11) boreholes.

Based on the SPT-N value and moisture content summarized in Table 7, clayey silt deposit is generally moist to very moist and is in very loose to dense condition, while the silty clay deposit is moist to wet and is in very soft to hard. The low SPT-N values for silty clay deposit is generally associated with high moisture content, and the high SPT-N is generally encountered at the interface between the silty clay deposit and weather shale.

Table 7: SPT-N and Moisture Content Summary for Silty Clay and Clayey Silt

Description	Min. Moisture Content (%)	Max. Moisture Content (%)	Min. SPT-N Value	Max. SPT-N Value
Clayey Silt deposit	12	23	3	32
Silty Clay deposit	8	33	0	50

Gradation analyses, summarized in Table 8, were carried out on three samples of clayey silt and four samples of silty clay deposit. Based on the gradation analysis results, the clayey silt deposit has silt content between 77% and 82% and clay content between 13% and 17%. The silty clay deposit has silt content between 46% and 70% and clay content between 29% and 46%.

Table 8: Gradation Analysis Result Summary for Silty Clay and Clayey Silt

BH No.	Depth (m)	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Description
BH15-4	2.29-2.9	0	10	77	13	Clayey Silt

BH No.	Depth (m)	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Description
BH15-5	1.52-2.13	0	6	77	17	Clayey Silt
MW2-13	0.9-1.4	0	4	82	14	Clayey Silt
MW1-13	5.3-5.9	0	5	49	46	Silty Clay
MW4-13	2.3-2.6	0	2	69	29	Silty Clay
MW5-13	5.3-5.9	1	7	46	46	Silty Clay
MW9-13	4.6-5.2	0	0	70	30	Silty Clay

Atterberg limit tests were carried out on three samples of clayey silt and four samples of silty clay, and the results are summarized in Table 9. According to the result, the plasticity index of the clayey silt deposit ranges from 3% to 8% indicating the low clay content.

The silty clay samples had plastic limit of 15% to 18%, liquid limit of 28% to 34 % and plasticity indices of 12% to 17% and were classified as low to medium plasticity clay. The moist content of the tested samples lied between the liquid and plastic limit and were described to be in moist to wet condition.

Table 9: Atterberg Limit Test Result Summary for Silty Clay and Clayey Silt

BH No.	Depth (m)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Moisture Content (%)	Description
BH15-4	2.3-2.9	15	18	3	23	Clayey Silt
BH15-5	1.6-2.1	14	6	8	9	Clayey Silt
MW2-13	0.9-1.4	24	16	8	20	Clayey Silt
MW3-13	3.8-4.4	22	17	5	22	Clayey Silt
MW1-13	5.3-5.9	33	17	16	27	Silty Clay
MW4-13	2.3-2.6	30	18	12	23	Silty Clay
MW5-13	5.3-5.9	34	17	17	28	Silty Clay
MW9-13	4.6-5.2	28	15	13	23	Silty Clay

5.2.4 Silty Clay Till

Silty clay till deposit was encountered below pavement structure in three boreholes (BH15-1, BH15-2 and BH15-3) extending to shale bedrock at depths from 5 m to 6.5 m below ground surface.

Based on the SPT-N values and moisture content summarized in Table 10, silty clay till deposit is generally moist and has a soft to hard consistency. The soft deposit was associated with high water content, and the hard deposit was encountered at the interface between silty clay till and shale bedrock.

Table 10: SPT-N Values and Moisture Content Summary for Silty Clay Till

Description	Min. Moisture Content (%)	Max. Moisture Content (%)	Min. SPT-N Value	Max. SPT-N Value
Silty Clay Till	16	20	4	50

Gradation analysis has been conducted on one representative sample, and the result is summarized in Table 11.

Table 11: Gradation Analysis Summary for Silty Clay Till

BH No.	Depth (m)	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Description
BH15-1	6.1-6.52	14	8	54	24	Silty Clay Till

Based on the Atterberg limit test result summarized in Table 12, the silty clay till was classified as low plasticity clay. The moist content of the tested sample lied close to its plastic limit and was described to be in moist condition.

Table 12: Atterberg Limit Test Result Summary for Silty Clay Till

BH No.	Depth (m)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Moisture Content (%)	Description
BH15-1	6.1-6.52	27	16	11	17	Silty Clay Till

5.2.5 Shale Bedrock

Shale bedrock was encountered in fifteen (15) boreholes. The weathered shale levels were determined by auger refusal or split spoon sampling, and rock coring with HQ size double tube wireline equipment was carried out in eight (8) boreholes to evaluate the rock quality with depth and to collect rock samples for laboratory testing. The weathered shale level and rock coring details are summarized in Table

13. Based on the borehole findings, weathered shale was encountered at depths ranging from 4.1 mbgs to 7.3 mbgs (from +80.1 masl to +75.1 masl).

Visual inspection on the rock cores indicates that the bedrock belongs to the Georgian Bay Formation consisting of highly weathered to fresh, grey to dark grey, fine to very fine-grained fissile shale, with occasional fresh, grey, fine grained calcareous siltstone and limestone layers. In addition, planes of weaknesses, including planes of fissility and bedding, contact surfaces between shale and siltstone or limestone bands and some oblique and subvertical joints, along the core were observed and tended to break. The joints along the bedding surfaces were occasionally infilled with clay, and the joints along the planes of fissility were generally smooth and clean. Detailed borehole logs and rock core pictures are provided in the project specific reports (CRA, February 2013 and Golder Associates, January 2015).

Table 13: Summary of Weathered Shale Level and Rock Coring

BH No.	Top of Weathered Shale		Start of Rock Coring		End of Rock Coring	
	Depth (mbgs)	Elevation (masl)	Depth (mbgs)	Elevation (masl)	Depth (mbgs)	Elevation (masl)
BH15-1	6.5	77.9*	NA	NA	NA	NA
BH15-2	6.3	78.5*	NA	NA	NA	NA
BH15-3	4.9	79.3*	NA	NA	NA	NA
BH15-4	4.7	79.3*	NA	NA	NA	NA
BH15-5	5.5	78.9*	NA	NA	NA	NA
MW2-13	6.4	75.1	7.3	74.6	10.5	74.4
MW3-13	6.9	78.0	7.4	77.4	10.6	74.2
MW4-13	4.1	80.1	5.2	79.0	8.2	76.0
MW5-13	6.4	78.6	7.2	77.8	15.4	69.7
MW6-13	6.6	79.6	7.3	78.9	10.4	75.8
MW7-13	6.9	79.6	7.3	79.4	10.5	76.2
MW8-13	5.3	78.6	5.6	78.4	13.9	70.1
MW8-13	6.9	77.6	7.4	77.0	10.6	73.9
BH1	7.0	NA	NA	NA	NA	NA
BH2	7.3	NA	NA	NA	NA	NA
* Borehole elevation was not provided in original borehole logs and was inferred from topographic plan and/or adjacent boreholes.						

According to the borehole logs, the recorded Rock Quality Designation (RQD) index in the upper portion of the bedrock (first rock core) varied between 0 and 75 percent, indicating the rock quality in the upper portion is in very poor to fair condition. The RQD values of the remaining rock cores varied between 30 and 100 percent, showing the lower portion of the bedrock is in poor to excellent condition. The Total Core Recovery (TCR) values varied between 70 and 100 percent.

Unconfined Compressive Strength (UCS) tests have been carried out on four (4) representative rock core samples, and the test results are summarized in Table 14.

Table 14: Summary of Unconfined Compressive Strength Test Result

BH No.	Rock Core Depth (mbgs)	Unconfined Compressive Strength (MPa)
MW1-13	7.54 to 7.65	30.2
MW4-13	7.16 to 7.28	28.9
MW5-13	11.9 to 12.0	108.9
MW9-13	9.27 to 9.40	35.2

5.3 Groundwater Conditions

Thirteen (13) groundwater observation wells were installed on site, and the well installation details, including well depth and screen levels, are summarized in Table 15.

As shown in Table 15, the measured groundwater level in the observation wells installed within the overburden soils ranged between 0.68 mbgs and 5.45mbgs (between +81.7 masl and +85.8 masl). However, the measured groundwater level in the monitoring wells installed in shale bedrock ranged between 7.94 mbgs and 11.53 mbgs (between +71.6 masl and +76.1 masl). Based on the available monitoring data, the groundwater table in shale bedrock was not hydraulically connected to the groundwater table in the overburden soils.

The project site is located near the Lake Ontario, which has an average water level at +74.4 masl. The groundwater gradient in the overburden soils is likely toward the Lake Ontario, and the groundwater table in the bedrock is likely influenced by the Lake Ontario.

The available monitoring data only covered a short period of time, which did not reflect the seasonal groundwater fluctuation.

Table 15: Summary of Measured Groundwater Readings

Well ID	Strata Screened	Top of Screen (mbgs)	Bottom of Screen (mbgs)	Measured Groundwater Reading		Date
				Elevation (masl)	Depth (mbgs)	
MW14-4	Silty Sand	1.8	3.7	81.8*	2.63	16 Dec 2014
MW14-6	Fill/Silt	0.6	2.9	82.7*	1.28	16 Dec 2014
MW1-13	Silt/Silty Clay	2.7	6.4	84.3	0.68	4 Mar 2013
MW2-13	Shale Bedrock	12.5	15.4	71.6	9.93	4 Mar 2013
MW3-13	Silt/Silty Clay	3.7	7.3	81.9	2.90	4 Mar 2013
MW4-13	Silty Clay/weathered Shale Bedrock	1.8	5.2	82.8	1.44	4 Mar 2013
MW5-13	Shale Bedrock	11.6	15.4	73.5	11.53	4 Mar 2013
MW6-13	Silt/Silty Clay	3.0	6.7	85.8	0.45	4 Mar 2013
MW7-13	Silty Clay	3.4	7.0	84.2	2.52	4 Mar 2013
MW8-13	Shale Bedrock	8.5	10.7	76.1	7.94	4 Mar 2013
MW9-13	Silt/Silty Clay	2.7	6.4	81.7	2.74	4 Mar 2013
BH1	Silt	6.1	9.1	NA	3.25	22 Oct 2004
BH2	Silt/Silty Clay	4.9	7.5	NA	5.45	22 Oct 2004
* Borehole elevation was not provided in original borehole logs and was inferred from topographic plan.						

5.4 Potential Contamination

Environmental Assessments for the site (Golder, 2019) highlights past land uses with potentially contaminating activities that included use or storage of the following: solvents, sulphuric acid, hydrocarbons, PCBs, and metals. The site has also been used for storage of ammunition from World War II and has included a sanitary landfill.

Following demolition of the Cookie factory structure, soil remediation was carried out in 2018 to target a number of identified contaminants, whilst further delineation of remaining contaminants was carried out. Remediation included excavation and removal of identified ‘hot spots’ and disused storage tanks. Groundwater quality testing was carried out as part of the Environmental Site Assessment and noted that the reported concentrations for contaminants discussed above were subsequently within applicable site condition standards (Golder, 2019).

6 Geotechnical Design Parameters

6.1 General

Geotechnical design parameters are interpreted based on the in situ and laboratory test results summarized in Table 16.

Table 16: Summary of In Situ and Laboratory Tests (min/average/max)

Soil Type	SPT-N Values	Moisture Content (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)
Granular Fill	4/14/27	4/8/16	NA	NA	NA
Earth Fill	0/12/34	7/16/42	NA	NA	NA
Clayey Silt	3/14/32	12/18/23	14/19/24	6/14/18	3/6/8
Silty Clay	0/11/50	8/20/33	28/31/33	15/17/18	12/14/16
Silty Clay Till	4/21/50	16/18/20	27	16	11

6.2 Strength Parameters

Due to the limited laboratory test result, the geotechnical design parameters are primarily interpreted based on empirical correlation presented in this section.

6.2.1 Unit Weight (γ)

Soil unit weight are determined based on laboratory testing (i.e. specific gravity) and empirical correlation with natural moisture content (w). The following correlation is adopted:

$$\gamma_{wet} = \frac{\gamma_w G_s (1 + w)}{1 + \left(\frac{w}{S}\right) G_s}$$

Where specific gravity, G_s equals 2.7 for cohesive soils and G_s equals 2.65 for cohesionless soils; Saturation (S) equals 0.9 and unit weight of water (γ_w) equals 9.81 kN/m³.

6.2.2 Friction Angle (ϕ) and Effective Cohesion (c')

For earth fill, friction angle is determined based on empirical correlation with corrected SPT N' values proposed by Peck, Hanson & Thornburn (1974), where $\phi' = 0.27 \cdot \text{SPT } N' + 27.5$.

For silty clay and clayey silt deposit, friction angle is determined based on empirical correlation with Plasticity Index proposed by Bjerrum and Simons (1960), where $\phi' = 0.0015 \text{PI}^2 - 0.29 \text{PI} + 36$.

For silty clay till, friction angle is determined based on empirical correlation with SPT N values proposed by Cao et al., 2015, where $\phi' = 32.5 + 0.09N$.

6.2.3 Undrained Shear Strength (S_u)

For normally consolidated to lightly overconsolidated silty clay deposit, the correlation of S_u with Plasticity Index (PI) proposed by Skempton and Henkel (1953) is adopted:

$$\frac{S_u}{\sigma'_{vc}} = 0.37 \text{PI} + 0.11$$

Where the ratio of S_u/σ'_{vc} is in kPa.

For over-consolidated silty clay till, the correlation of S_u with PI proposed by Stroud and Butler (1975) is adopted:

$$s_u = N \times [5 \times 10^{-5}(\text{PI})^3 + 0.008(\text{PI})^2 - 0.41(\text{PI}) + 10.6]$$

6.2.4 Proposed Geotechnical Design Parameters

The proposed geotechnical design parameters are summarized in Table 17.

Table 17: Summary of Geotechnical Design Parameters

Soil Type	Bulk Unit Weight	Effective Friction Angle	Effective Cohesion	Undrained Shear Strength	Unfactored Friction Coefficient*
	γ (kN/m ³)	ϕ' (°)	c' (kPa)	S_u (kPa)	μ
Granular Fill	20	32	0	0	0.39
Earth Fill	18	28	0	0	0.34
Clayey Silt	19	28	0	0	0.34
Silty Clay	19	25	0	50	0.30
Silty Clay Till	22	34	0	100	0.42
* Unfactored friction coefficient is determined by the friction angle, $\mu = \tan(2/3\phi)$					

6.3 Hydraulic Conductivities

The anticipated permeability for the geological units are determined based on descriptions within boreholes logs, available particle size distribution data, and experience of similar ground conditions in the area. Anticipated permeabilities are summarized in Table 18.

Table 18: Anticipated Permeability for Geological Units

Strata	Anticipated Strata Thickness (m)	Typical Hydraulic Conductivity (m/s)
Fill	2.3	1×10^{-6} to 1×10^{-7}
Old Lake Deposits (silty clay/clayey silt)	5.5	1×10^{-7} to 1×10^{-8}
Till (silty clay/clayey silt)	3.0	1×10^{-9} to 1×10^{-8}
Upper Weathered/Fractured Shale	0.7	1×10^{-7} to 1×10^{-8}
Shale	>50m*	1×10^{-10} to 1×10^{-8}
Note: *Thickness of the shale has not been proven, however it is expected that shale formations (with limestone) will have a thickness greater than 50 m.		

7 Discussion and Recommendations

7.1 General

The purpose of this report is to summarize the ground and groundwater conditions based on the available geotechnical investigation and to provide assessment on the soil, bedrock and groundwater characteristics of the subject property to determine its feasibility and stability to accommodate the proposed development.

The following sections provide the preliminary design and construction recommendations for site preparation, foundations, floor slabs, retaining walls, temporary shoring system, underground services, pavement structure, earthquake consideration and dewatering for the proposed development. In addition, this report provides discussion on potential risks, mitigation measures and monitoring programs for the proposed development.

At the time of this report preparation, details of the proposed development are under development; as a result, the preliminary recommendations provided herein should be reviewed and revised during future design stages of the project when additional information is available.

7.2 Site Preparation

The existing topsoil, asphalt, surficial soils and earth fill with organic materials are considered not suitable as subgrade material to support the proposed pavement structure, foundations, slab-on-grade, engineered fill or any settlement sensitive structures and should be removed prior to construction of these structures.

The existing earth fill and granular fill with no organic or deleterious matters may be reused as engineered fill for regrading. Additional laboratory testing on the existing granular fill, i.e. gradation analysis, is required to verify its quality prior to be reused as subbase materials for the proposed structures.

Existing underground services and infrastructure on site may need to be decommissioned, removed and properly backfilled with suitable backfill materials.

All subgrade materials shall be proof-rolled under supervision by the geotechnical engineer. Any wet, soft or loose subgrade materials shall be replaced with suitable backfill materials and properly compacted.

In general, fill materials will be in accordance with OPSS 1010, and the fill compaction requirement will be in accordance with OPSS 501 and SSP501SS2.

7.3 Foundations

All geotechnical structures such as foundations and retaining walls shall be designed according to the limit state design approach, which is set forth in the

Ontario Building Code, Canadian Highway Bridge Design Code, and other such documents. In this approach, the factored resistance must equal or exceed the factored load demand to satisfy the Ultimate Limit State (ULS). Furthermore, the unfactored load demand must not exceed a nominal unfactored resistance commensurate with an allowable degree of movement or settlement such that the Serviceability Limit State (SLS) is satisfied. Hence for foundations such as spread footings and drilled shafts, both ULS and SLS resistances are provided.

7.3.1 Mat Foundation/Spread Footings

Based on the Master Plan, the proposed development includes low, mid and high-rise buildings, fifteen towers ranging in height from 22 to 71 storeys with up to 5-level basement car park. Mat foundation or spread footings may be employed to support columns on square or rectangular pad foundations or continuous walls on strip foundations. Depending on the detailed location of basements, these foundations may bear either on the underlying shale bedrock, upper overburden soils, or shallow engineered fill. The existing earth fill is not considered as suitable subgrade material for foundation construction. SLS and factored ULS bearing pressures are provided in Table 19 as a function of bearing material, footing geometry and founding depth.

It is recommended that a consistent bearing material be employed for all foundations to minimize differential settlements. Foundations with different bearing materials shall be separated by a construction joint in the slab and superstructure, to allow for each portion of the structure to move independently of the other.

Table 19: Summary of SLS and ULS Bearing Capacities for Geological Units

Description	SLS Bearing Capacity (kPa)	ULS Bearing Capacity (kPa)	Note
Clayey Silt/Silty Clay	75	100	The bearing capacities are based on strip footing dimensions of 0.45m W by 10m L or spread footings with a maximum area of 1m ² .
Silty Clay Till	250	350	
Weathered Shale	1,200	1,800	Minimum embedment of 0.6m into shale.

7.3.2 Deep Foundation

Where large concentrated loads are to be applied, bored pile foundations drilled into the underlying shale bedrock would be appropriate for this site. The minimum rock socket length shall be the larger of 1,200 mm or two pile diameters into the shale. It is estimated that 1 m diameter bored piles would have SLS capacities of 3 MN and factored ULS axial capacities of 4.5 MN, based primarily on the allowable compressive strength of concrete and the underlying shale bedrock. Final capacities

are a function of concrete compressive strength, rock socket length (depth into rock), and results of pile load testing.

Due to the shallow groundwater table in the overburden soils, construction methods for such piling shall employ either a temporary steel casing to top of rock or construction under bentonite or polymer slurry (drilling fluid) to provide temporary stabilization of pile walls through the overburden prior to tremie concreting.

An alternative to bored piling would be driven piling, either H-piles or concrete-infilled driven pipe piles, driven to refusal in the glacial till or shale bedrock, with capacity limited by the allowable driving stress during pile installation. For example, a 325 mm O.D. driven steel pipe pile, later infilled with concrete, is estimated to have an SLS capacity on the order of 1 MN and factored ULS capacity on the order of 1.5 MN. Final capacities are a function of pile wall (steel) thickness, pile hammer weight and drop height, measured driving stress, refusal criteria, and pile load test results.

7.3.3 Frost Protection and Shale Bedrock Protection

All pad or strip foundations shall either have a 1.2 m soil cover, or employ insulation, to protect against frost heave. All foundation subgrade must be protected against frost during winter construction. Frost protection, i.e. extruded polystyrene insulation, shall be provided around ventilation shafts, ramp slabs and ramp walls leading into underground structures, where ambient temperatures remain below zero for extended periods.

Shale bedrock may weather rapidly between wetting and drying cycles; therefore, where necessary, exposed shale bedrock surfaces may be protected with lean concrete mat.

7.4 Slab-on-grade

The existing topsoil, surficial soils and earth fill with organic materials are not considered as suitable subgrade materials for slab-on-grade construction and should be removed. The existing clean earth fill in its current state is also not suitable for slab-on-grade construction but can be excavated and properly recompacted as engineered fill for slab-on-grade construction.

Subgrade must be proof rolled prior to placement of engineered fill. Any wet, soft and loose materials shall be replaced with suitable backfill material and properly recompacted. Earth borrow or Granular B in accordance with OPSS 1010 can be used to raise the site grade and shall be compacted in accordance with OPSS 501.

Where groundwater level is within 2 m below the slab subgrade level, drainage system shall be provided beneath the slab and shall be connected to positive outlet. Floor slabs constructed below groundwater level shall be designed for hydraulic uplifting force with water proofing measures.

7.5 Earth Retaining Structures

7.5.1 Shoring System

High-rise buildings, fifteen towers ranging in height from 22 to 71 storeys, with up to 5-level basement car park is proposed for the development, which will require a basement excavation down to the Georgian Bay Shale level at approximately +67 masl. Shoring system consisting of secant pile wall can be considered for the proposed excavation. Secant pile wall can reduce the amount of dewatering and the associated impact on adjacent structures. Soldier pile and lagging wall can be considered as alternative shoring system; however, high water ingress to the excavation is anticipated due to the shallow groundwater and appropriate dewatering system shall be designed to control water seepage into the excavation.

Soil-structure interaction modelling is recommended to be carried out for each stage of the excavation to determine the loading on the structural elements and anticipated deformation of the shoring system as well as stability at each excavation stage. The appropriate surcharge loading from the construction activities, surrounding structures or traffic as well as the hydro-static pressure shall be incorporated into the analysis.

Secant piles or soldier piles shall be socketed into the sound bedrock in accordance with the result of the soil-structure interaction modelling to provide adequate stability for the shoring system. Temporary casing shall be provided for piling to prevent overburden soils caving into the drilled hole.

The proposed excavation will intercept both upper (between elevation +81.7 masl and +85.8 masl) and lower (between elevation +71.6 masl and +76.1 masl) groundwater levels. Further hydrogeological study and groundwater monitoring covering seasonal fluctuation is recommended to determine the appropriate design groundwater level and distribution for the shoring design.

Soil and rock anchors can be considered to provide lateral support to the shoring system. Anchor design and load tests (pre-production and production tests) shall be carried out in accordance with Canadian Foundation Engineering Manual (CFEM) and PTI DC35.1-14.

7.5.2 Retaining Walls

Retaining walls up to 7.5 m in height are anticipated for the current design of the proposed relief road (Great Northern Gateway, GNG), which runs along the northern site boundary, broadly parallel to the Gardiner East Lake Shore West Ramp. Due to the proximity to the existing road ramp, the conventional cantilever rigid retaining wall (Inverse-T wall) requiring large excavation for construction is not recommended; alternatively, secant pile wall can be considered for the proposed relief road construction.

Significant wall deflection is anticipated during excavation with cantilever secant pile wall located in proximity to the existing roadway ramp, which may result in potential surface settlement on the existing roadway ramp behind the retaining wall. Soil or rock anchors can be installed to restrain the wall deflection and limit the influence on the existing roadway; however, property easement may be required to allow anchor installation. Anchor design and load tests (pre-production and production tests) shall be carried out in accordance with Canadian Foundation Engineering Manual (CFEM) and PTI DC35.1-14

Soil-structure interaction modelling is also recommended to be carried out for each stage of the excavation to determine the loading on the structural elements and anticipated deformation of the secant pile wall as well as stability at each excavation stage. The appropriate loading from the surrounding structures and/or traffic as well as the hydro-static pressure shall be incorporated into the analysis.

7.5.3 Lateral Earth Pressure

Shoring system, basement walls and similar earth retaining structures shall be designed to support lateral earth pressure and hydrostatic pressure, which can be calculated by the following:

$$P = K[\gamma(h-h_w) + \gamma'h_w + q] + \gamma_w h_w$$

Where:	P	=	horizontal pressure at depth, h (m)
	K	=	lateral earth pressure coefficient (Table 20)
	γ	=	bulk unit weight of soil, (kN/m ³)
	γ'	=	submerged unit weight of soil, (kN/m ³)
	γ_w	=	unit weight of water, (kN/m ³)
	h_w	=	depth below groundwater level (m)
	q	=	surcharge load (kPa)

The proposed lateral earth pressure coefficients for the geological units are summarized Table 20.

Table 20: Summary of Lateral Earth Pressure Coefficient

Soil Type	Bulk Unit Weight	Effective Friction Angle	Lateral Earth Pressure Coefficient		
	γ (kN/m ³)	ϕ' (°)	k_a	k_0	k_p
Granular Fill	20	32	0.31	0.47	3.25
Earth Fill	18	28	0.36	0.53	2.77
Clayey Silt	19	28	0.36	0.53	2.77
Silty Clay	19	25	0.41	0.58	2.46
Silty Clay Till	22	34	0.28	0.44	3.54

7.6 Excavation

The design of temporary shoring of trench excavations for utilities, structural footings and slabs shall be the responsibility of the contractor and shall conform to all applicable codes and guidelines.

Specifically, OHSA regulations require that all excavations 1.2m and deeper must be sloped and/or braced in accordance with OHSA requirements. OSHA divides soils into four different types as defined in Table 21.

Table 21: Soil Definition based OHSA

Soil Type	Definition
1	<ul style="list-style-type: none"> a) Hard, very dense and only able to be penetrated with difficulty by a small sharp object; b) Has a low natural moisture content and a high degree of internal strength; c) Has no signs of water seepage; and, d) Can be excavated only by mechanical equipment.
2	<ul style="list-style-type: none"> a) Very stiff, dense and can be penetrated with moderate difficulty by a small sharp object; b) Has a low to medium natural moisture content and a medium degree of internal strength; and, c) Has a damp appearance after it is excavated.
3	<ul style="list-style-type: none"> a) Stiff to firm and compact to loose in consistency, or is previously excavated soil; b) Exhibits signs of surface cracking; c) Exhibits signs of water seepage; d) If it is dry, may run easily into a well defined conical pile; and, e) Has a low degree of internal strength.
4	<ul style="list-style-type: none"> a) Soft to very soft and very loose in consistency, very sensitive and upon disturbance is significantly reduced in natural strength; b) Runs easily or flows, unless it is completely supported before excavating; c) Has almost no internal strength; and, d) Exerts substantial fluid pressure on its supporting system, per Ontario Regulation 213/91 s226(5).

In relation to the OHSA soil classification, onsite soils which are dry are generally classified as Type 3, whereas damp/moist to wet soils shall be classified as Type 4.

The trenching requirements based on OHSA regulations are summarized in Table 22.

Table 22: Excavation Side-Slope Gradient based on OHSA

Soil Type	Base of Slope	Maximum Slope Inclination
1 & 2	Vertical within 1.2m from bottom of excavation	1 Horizontal to 1 Vertical
3	From bottom of excavation	1 Horizontal to 1 Vertical
4	From bottom of excavation	3 Horizontal to 1 Vertical

Where site geometry does not allow for the required backslopes of 1:1 or 1:3 (Type 3 and 4 soils, respectively), a braced excavation shall be designed according to lateral earth pressure, discussed in Section 7.5.1, OHSA Regulations and OPSS 539 performance level 2. Where existing adjacent structures are located within the active wedge of the excavation, OPSS 539 performance level 1A is to be adopted.

Based on the available groundwater monitoring data, shallow groundwater table is expected, and temporary dewatering is anticipated for the proposed excavation.

Local construction experience in the Georgian Bay Formation indicates that methane gas could be encountered during excavation in the bedrock; therefore, potential mitigation measures, i.e. monitoring and ventilation system, may be required during excavation. However, no methane gas was detected during project specific geotechnical investigation.

7.7 Underground Services

Temporary excavation for underground utilities installation shall be carried out in accordance with Section 7.5.

Based on the revealed subsurface condition, the subgrade for the proposed underground utilities will likely consist of clayey silt, silty clay or silty clay till, which are generally considered as suitable subgrade material for the proposed utility construction. Where organic/deleterious materials and soft or loose deposits were encountered at the subgrade level, they shall be replaced with suitable backfill materials and properly recompacted prior to placement of bedding.

For City of Toronto utilities, fills will be in accordance with TS 1010 and TS 401 where all embedment and bedding material are to be Granular A or Granular A RCM. Cover material are to be Granular A or B. Backfill material will be Granular A, Granular A RCM, unshrinkable fill or native material. Details for utility trench backfill are shown in Figure 4.

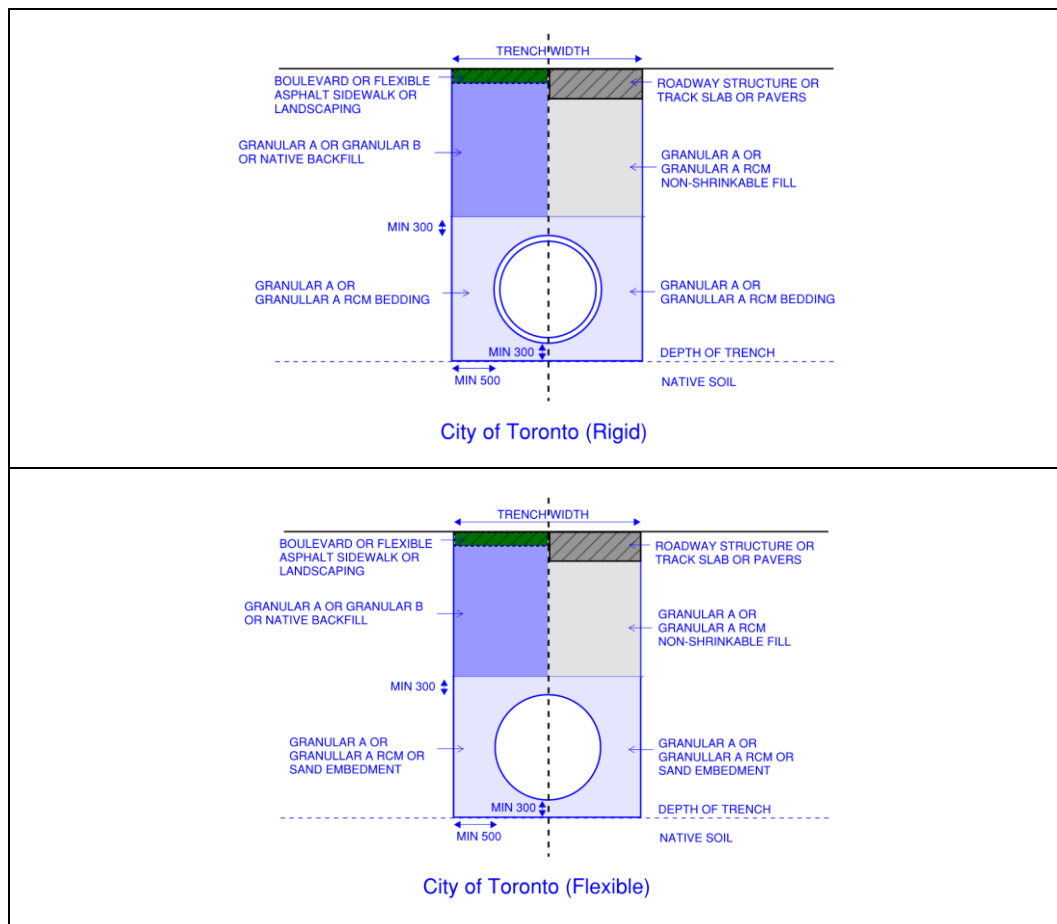


Figure 4: Backfill for Utility Trenches

7.8 Pavement Structure

The existing topsoil, surficial soils and earth fill with organic materials are not considered as suitable subgrade materials for pavement construction and should be removed.

The existing earth fill free of organic and deleterious materials is considered as suitable subgrade for the pavement construction. Subgrade material must be proof-rolled prior to pavement construction. Any wet, soft and loose materials shall be replaced with suitable backfill material and properly recompacted.

Earth borrow or Granular B in accordance with OPSS 1010 can be used to raise the site grade and shall be compacted in accordance with OPSS 501 and SSP501SS2.

The recommended pavement design for the proposed roadways and parking areas is summarized in Table 23.

Table 23: Summary of Pavement Design

Pavement Layer	Compaction Requirements	Light Duty Pavement Structure	Heavy Duty Pavement Structure
Surface Course Asphaltic Concrete HL3 (OPSS 1150)	91% to 96.5% Maximum Relative Density (OPSS 310)	40 mm	40 mm
Base Course Asphaltic Concrete HL8 (OPSS 1150)	92% to 97.5% Maximum Relative Density (OPSS 310)	40 mm	60 mm
Base Course: Granular A or 19mm Crusher Run	100% Standard Proctor Maximum Dry Density	150 mm	150 mm
Sub-base Course: Granular B or 50mm Crusher Run	98% Standard Proctor Maximum Dry Density	250 mm	350 mm

7.9 Dewatering

Based on the available monitoring data, the groundwater table in the overburden soils ranged between 0.68 mbgs and 5.45mbgs (between +81.7 masl and 85.8 masl), and the measured groundwater level in shale bedrock ranged between 7.94 mbgs and 11.53 mbgs (between +71.6 masl and +76.1 masl). The proposed 5-level basement will extend approximately to +67 masl, below both upper (perched) and lower groundwater tables.

A more comprehensive hydrogeological study, i.e. in-situ testing and modelling, shall be carried out to verify the hydrogeological behaviours and characteristic of the geological units, to select the appropriate shoring system, i.e. secant pile wall or soldier pile and lagging wall, dewatering system and to determine the anticipated volume of water for permits approval.

Ministry of Environment (MOE) Permit to Take Water (PTTW) will be required where dewatering volume exceeds 50,000 L/days. In addition, Private Water Discharge permit is also required by City of Toronto to discharge water to the municipal storm sewer system. The volume of dewatering is governed by the groundwater level, hydrogeological characteristics of the geological units, and the shoring system.

Where necessary, the dewatering system shall be designed to allow stable sides and bottom for the proposed excavation during the construction period taking the groundwater fluctuation into account. Adequate filters shall be provided to prevent fine grain soils migration due to pumping. Contingency pumping system shall be available in case of emergency. Gradual groundwater recovering period shall be allowed to prevent fine grain soils migration.

7.10 Seismic Site Class

The seismic site class is a function of the average shear wave velocity (V_s), SPT N-Value, or soil undrained shear strength (S_u) within the top 30m of the soil profile, as shown in Table 24. For mat/slab foundations bearing directly on the shale bedrock, Site Class C (very dense soil / soft rock) may be adopted. For foundations bearing on the overburden soils, Site class D (stiff soil) may be more appropriate. It should be noted that the above seismic site class is only preliminary, and a quantitative assessment for proper determination of seismic site class is required by the seismic provision of NBCC. Multi-channel analysis of surface waves (MASW) is recommended to obtain the shear wave velocity of the soil and rock profile for the quantitative assessment to determine the proper seismic site class.

Table 24: Seismic Site Class

Site Class	Type of Soil Profile	Average Properties in Top 30m		
		Soil Shear Wave Average Velocity V_s (m/s)	Standard Penetration Resistance N_{60}	Soil Undrained Shear Strength S_u (kPa)
A	Hard Rock	$V_s > 1500$	-	-
B	Rock	$760 < V_s < 1500$	-	-
C	Very Dense Soil and Soft Rock	$360 < V_s < 750$	$N_{60} > 50$	$S_u > 100$
D	Stiff Soil	$180 < V_s < 360$	$15 \leq N_{60} \leq 50$	$50 < S_u < 100$
E	Soft Soil	$V_s < 180$	$N_{60} < 15$	$S_u < 50$
F	Others	Site Specific Evaluation Required		

According to the Supplementary Standard SB-1 of the 2006 OBC, for the Toronto Area the mapped Spectral Response Acceleration (S_a) value of 0.26g (PGA) for short duration (period) of 0.2 second and 0.055g for one second duration. These parameters should be reviewed by the structural engineer.

7.11 Monitoring

Excavations adjacent to existing infrastructure, primarily the railway and Gardener Expressway corridors, shall be monitored for movement. The combined use of fixed survey points and inclinometers (either embedded in the retaining structures themselves or nearby in either the retained soil or passive zone) is recommended.

The allowable movements shall be established by the owners (Metrolinx and City of Toronto, respectively). Sample values of allowable, review, and alarm movement

limits from Metrolinx are provided in Table 25. Allowable movements reported by MTO for the RER track widening project beneath MTO's 401/409 highway are on the order of 20 to 25mm. Application of these values to the project at hand shall be confirmed with the respective owners prior to commencement of excavation and monitoring.

Table 25: Summary of Allowable, Review and Alarm Levels (Metrolinx)

Class of Track	Allowable Limits (mm)		Review Limits (mm)		Alarm Limits (mm)	
	Horizontal	Vertical	Horizontal	Vertical	Horizontal	Vertical
1/yard	0-10	0-12	10-15	12-20	>15	>20
2	0-4	0-4	4-9	4-12	>9	>12
3						
4						
5						

Temporary excavations shall be constructed following OPSS 539 – Construction Specification for Temporary Protection Systems and shall be designed to at least performance level 2 as described in Table 26, with a maximum settlement of 25mm and angular distortion of 1:200. Again, these criteria should be verified with third party owners.

Table 26: Performance Levels for Temporary Shoring System (OPSS 539)

Performance Level	Maximum Angular Distortion	Maximum Horizontal Displacement (mm)
1a	1:1000	5
1b	1:1000	10
2	1:200	25
3	1:100	50

8 Further Work

This report is based on previous ground investigation work carried out at the site and a review of historical records within and surrounding the site. Although ground investigations have been carried out within the site, ground investigation has yet to be completed specifically for the current proposed development. As a result, analysis in this report is based on available field and lab testing and should be considered preliminary for initial recommendations. Gaps in the available data for further design stages will be addressed by further site investigation and analysis. The results of these further investigations can be provided to the City of Toronto if required.

References

Canadian Foundation Engineering Manual (2006). Canadian Geotechnical Society.

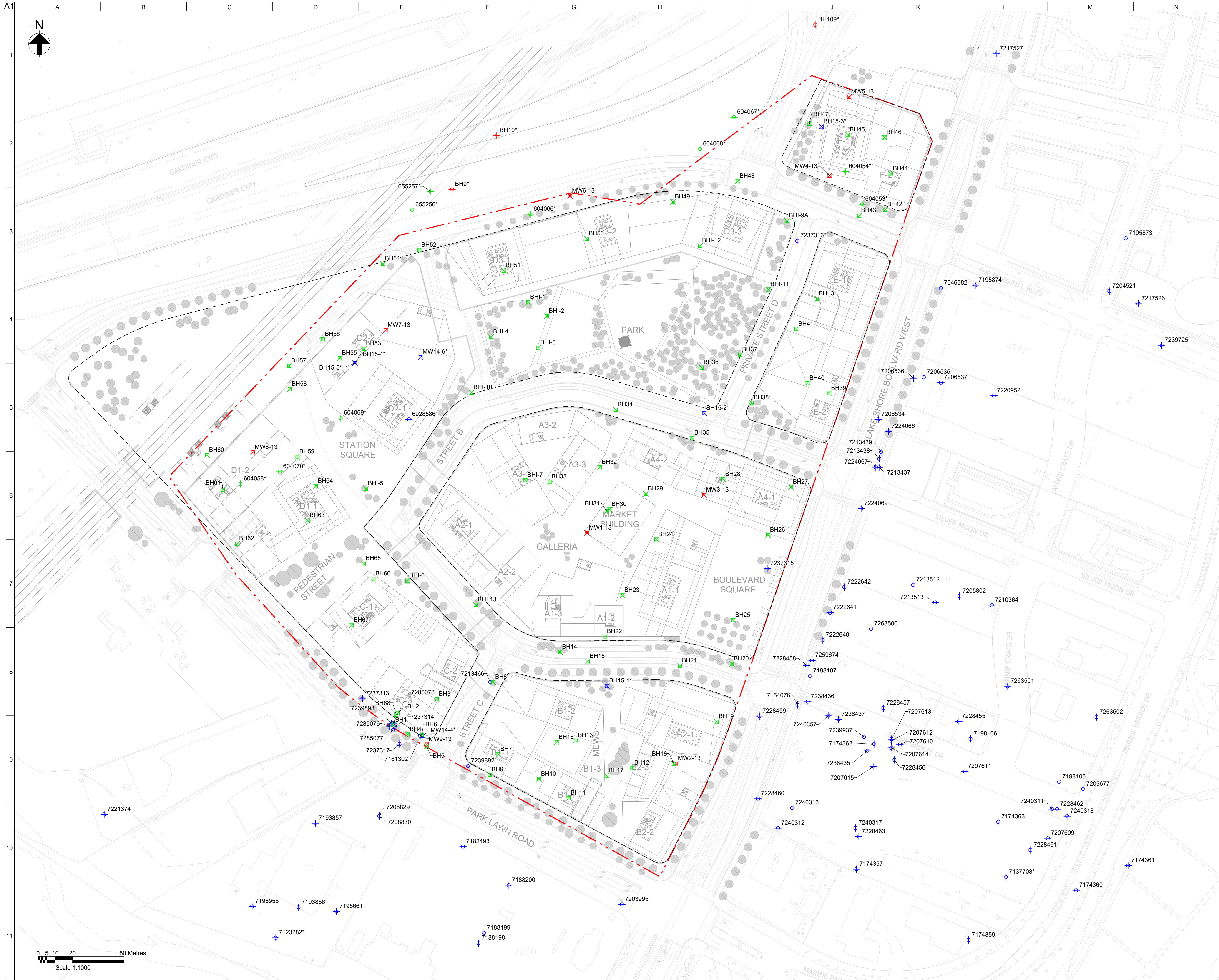
Etobicoke Historical society. Humber Bay.
<http://www.etobicokehistorical.com/humber-bay.html>

National Building Code of Canada (2015). National Research Council Canada.

Ontario Geological Survey (1980), Quaternary Geology of Toronto and the Surrounding Area. Ministry of Natural Resources Ontario,

Appendix A

Borehole Location Plan



- Notes**
1. All coordinates are in metres and referenced to Zone 10 of the Ontario Coordinate System, NAD (North American Datum) 1927, 1968 adjustment, 3 degree MTM (NAD 27 MTM 3 degrees Zone 10).
 2. All dimensions are in metres unless noted otherwise.
 3. This drawing represents 3rd party data and it has been produced for information only, to orientate the geotechnical and hydrological studies for this development.

- Legend**
- Site Boundary
 - Borehole Location
Source: Golder (2014)
 - Borehole Location
Source: CRA (2013)
 - Borehole Location
Source: SPL (2013)
 - Borehole Location
Source: MTO Foundations Library
 - Borehole Location
Source: Ontario Well Database
 - Borehole Location
Source: Ontario Geotechnical Boreholes Database

NOTE:
THIS DRAWING CONTAINS EXTENSIVE USE OF COLOUR
LINE WORK AND MUST BE PLOTTED IN COLOUR.

01	15/05/20	AR	LS	HJ
For planning submission				
Rev	Date	By	Chkd	Appd

ARUP

Arup Canada Inc.
121 Bloor Street East
Suite 900
Toronto ON M4W 3M5
Canada
www.arup.com

Client

FCR (Park Lawn) LP and
CPPIB Park Lawn Canada Inc.

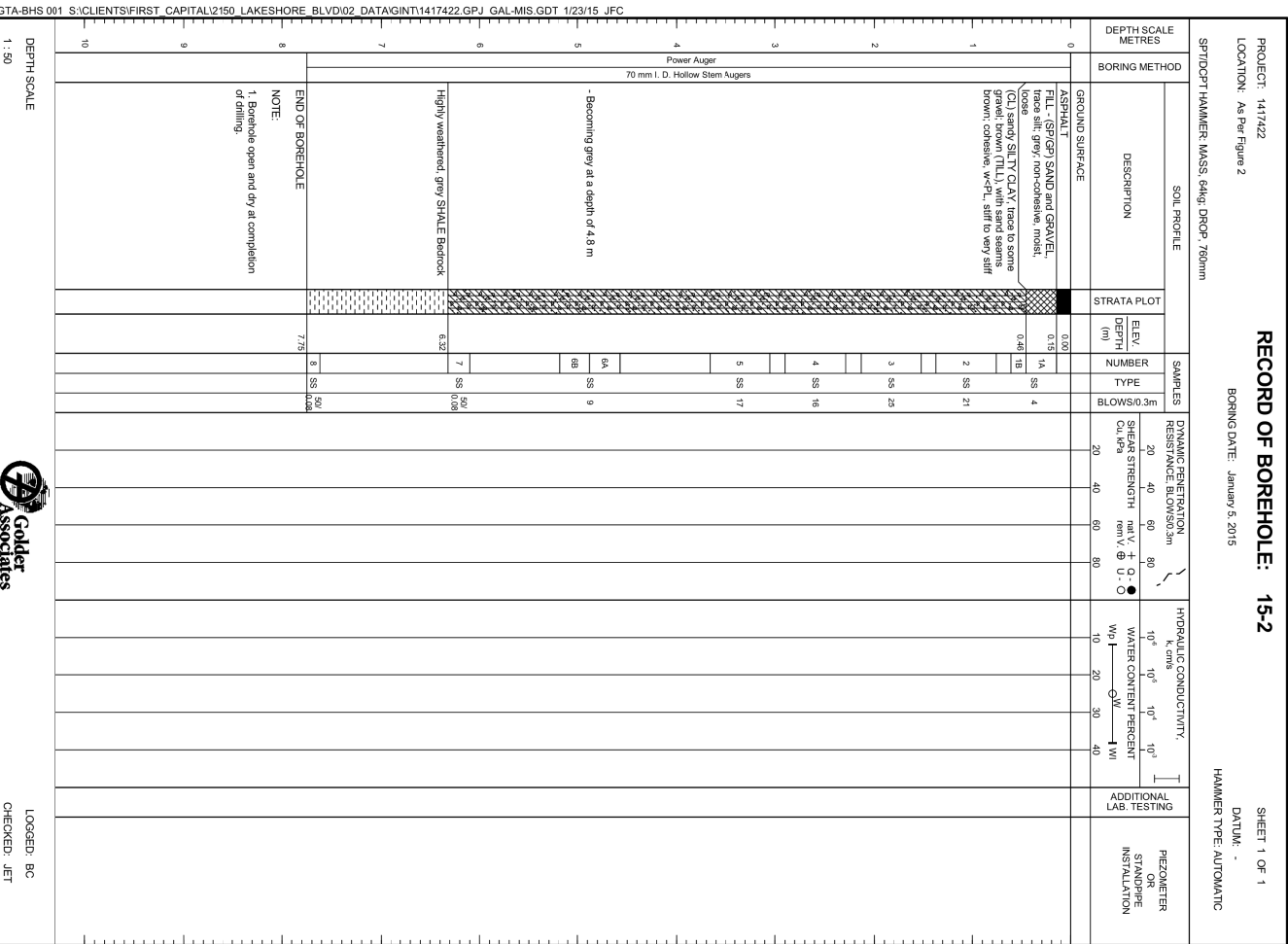
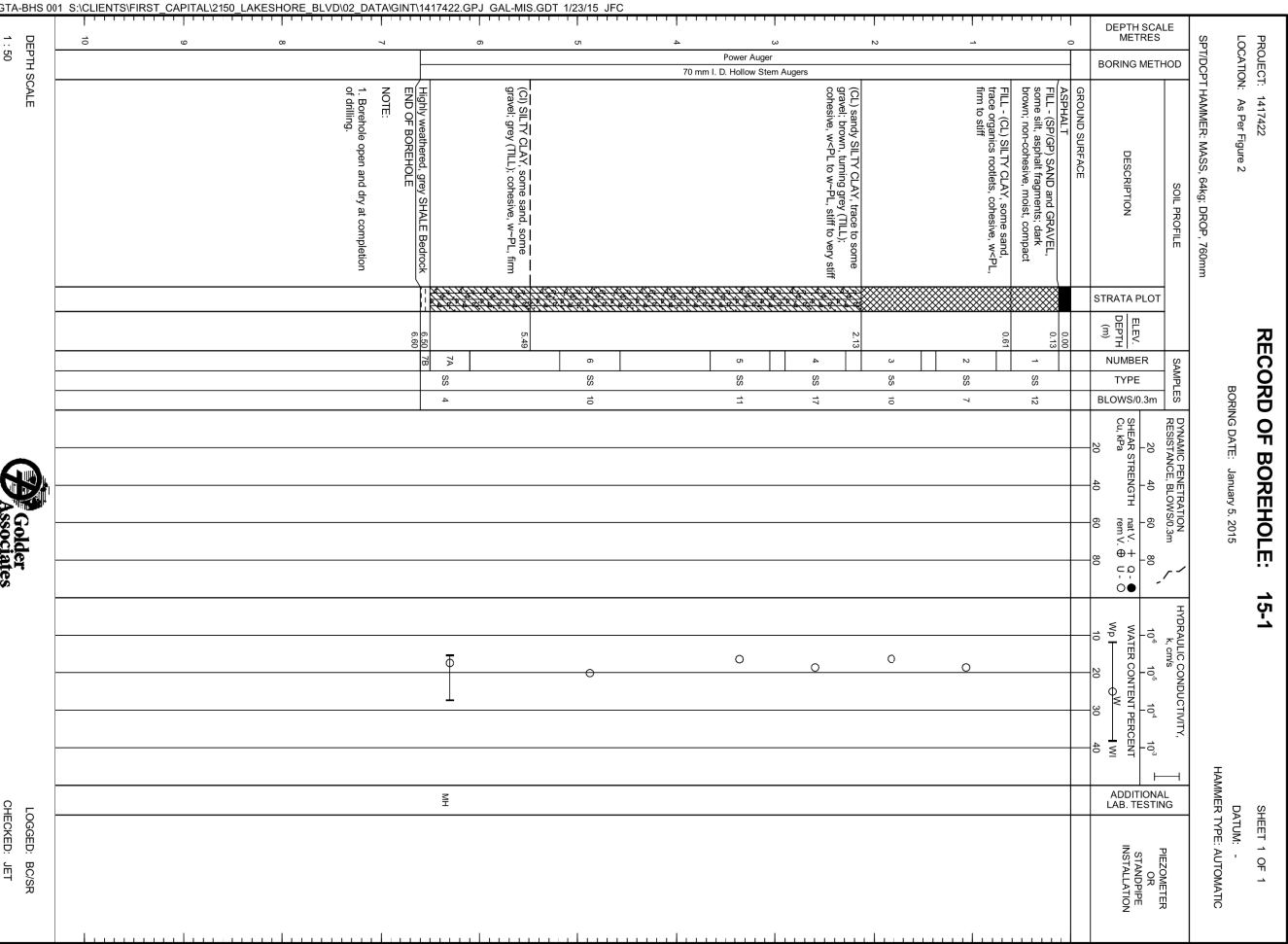
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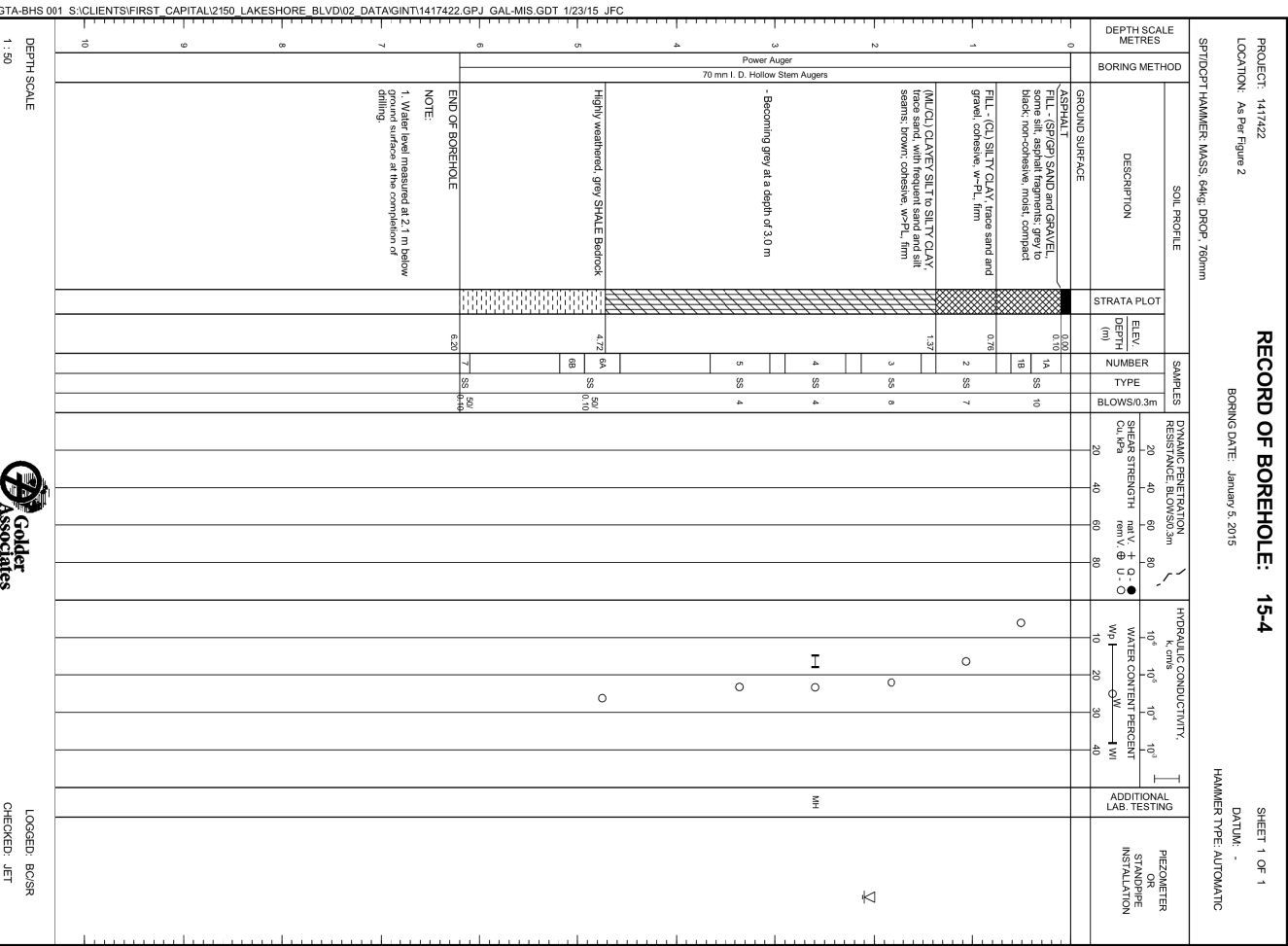
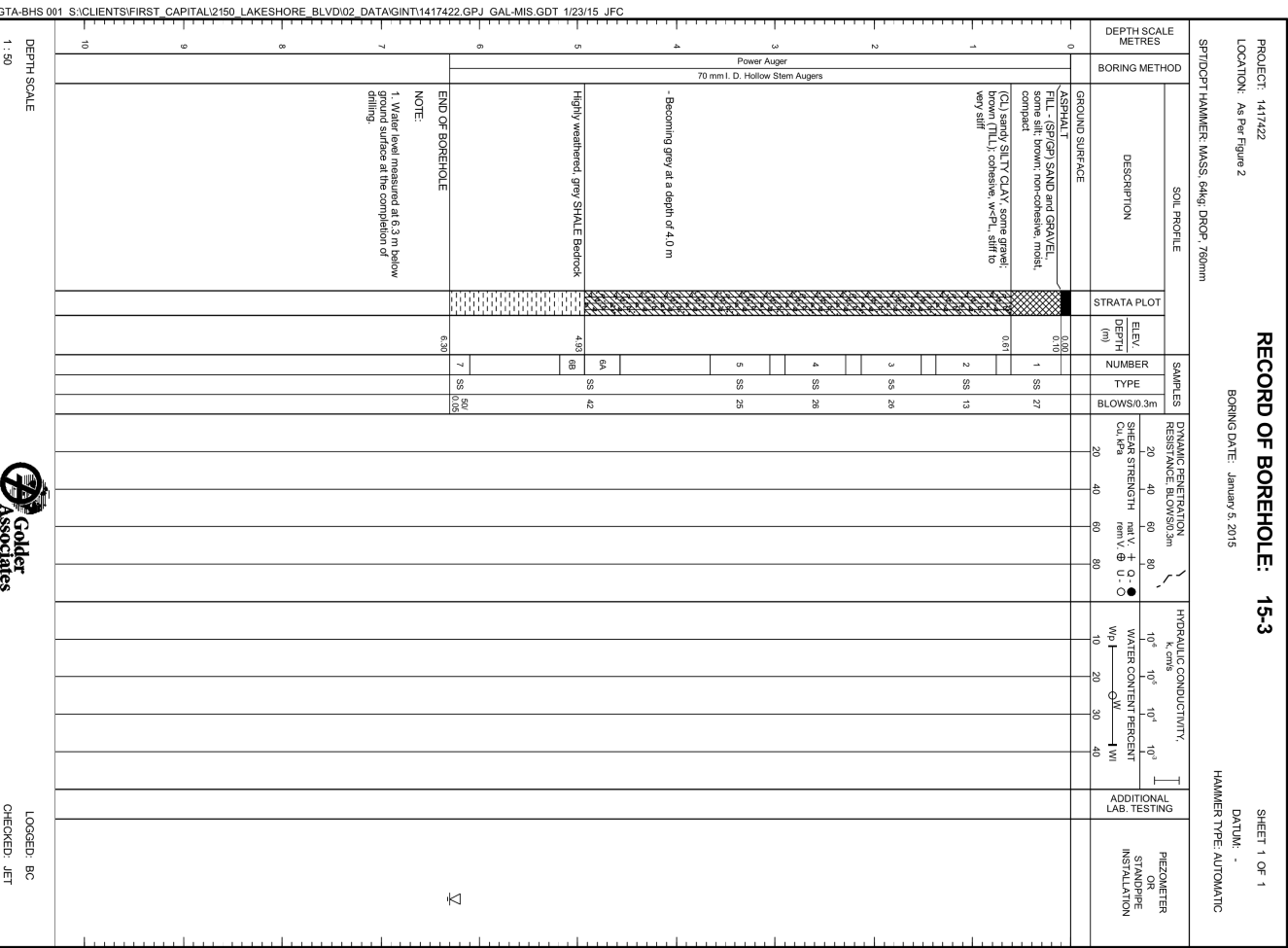
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Borehole Location Plan

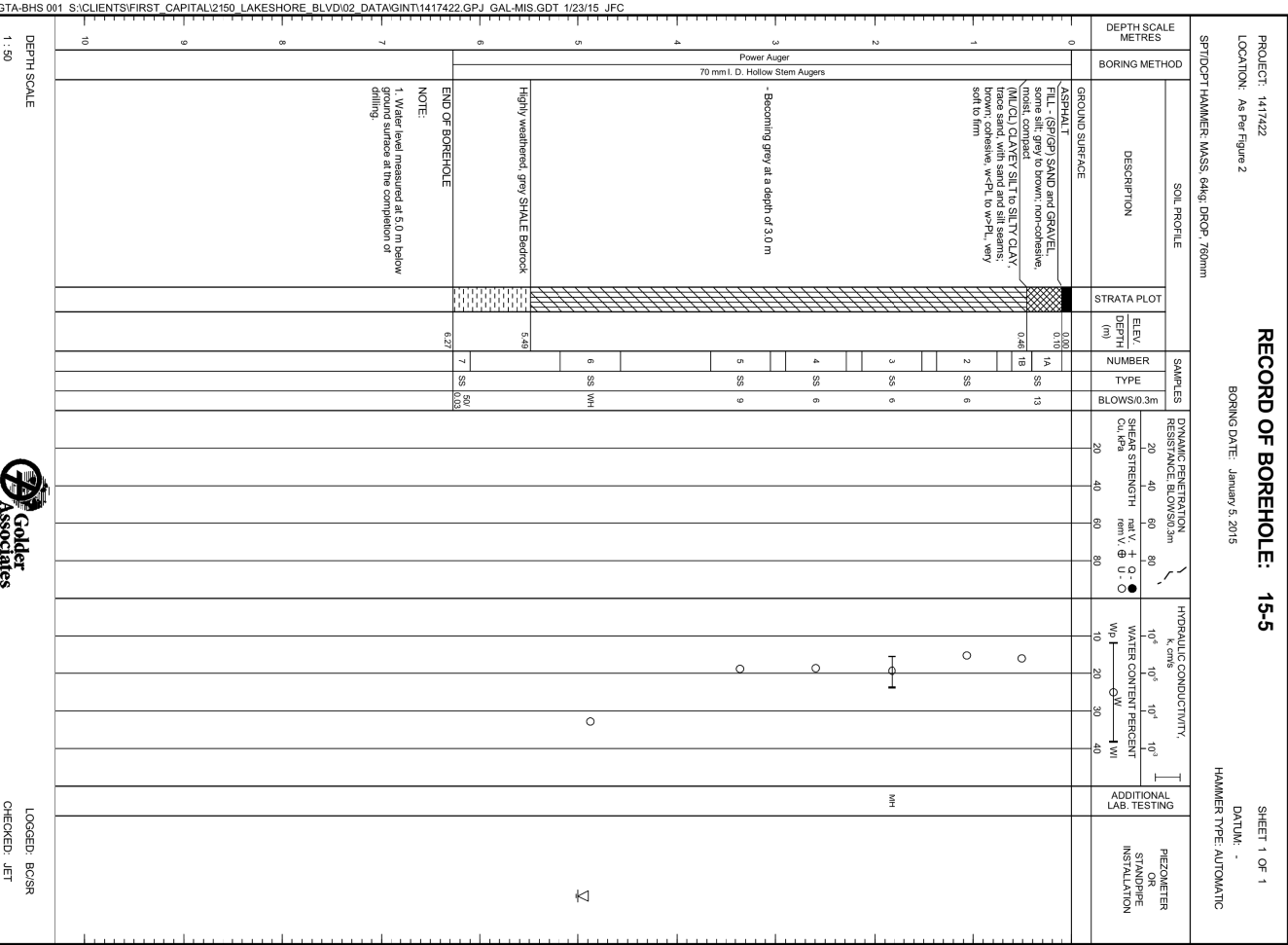
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Role	Geotechnics
Suitability	Detailed Masterplan
Arup Job No	Rev
264635	01
Name	
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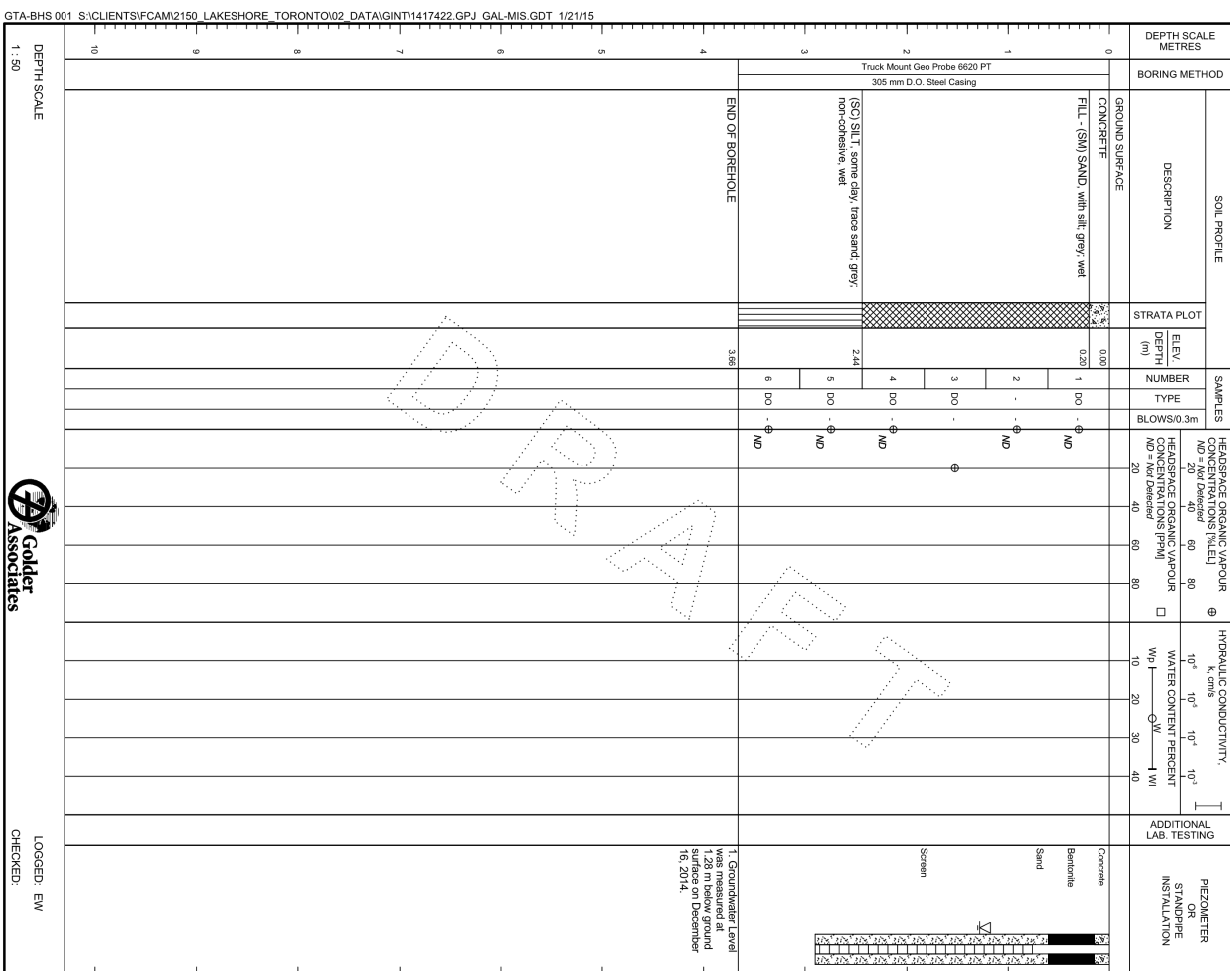
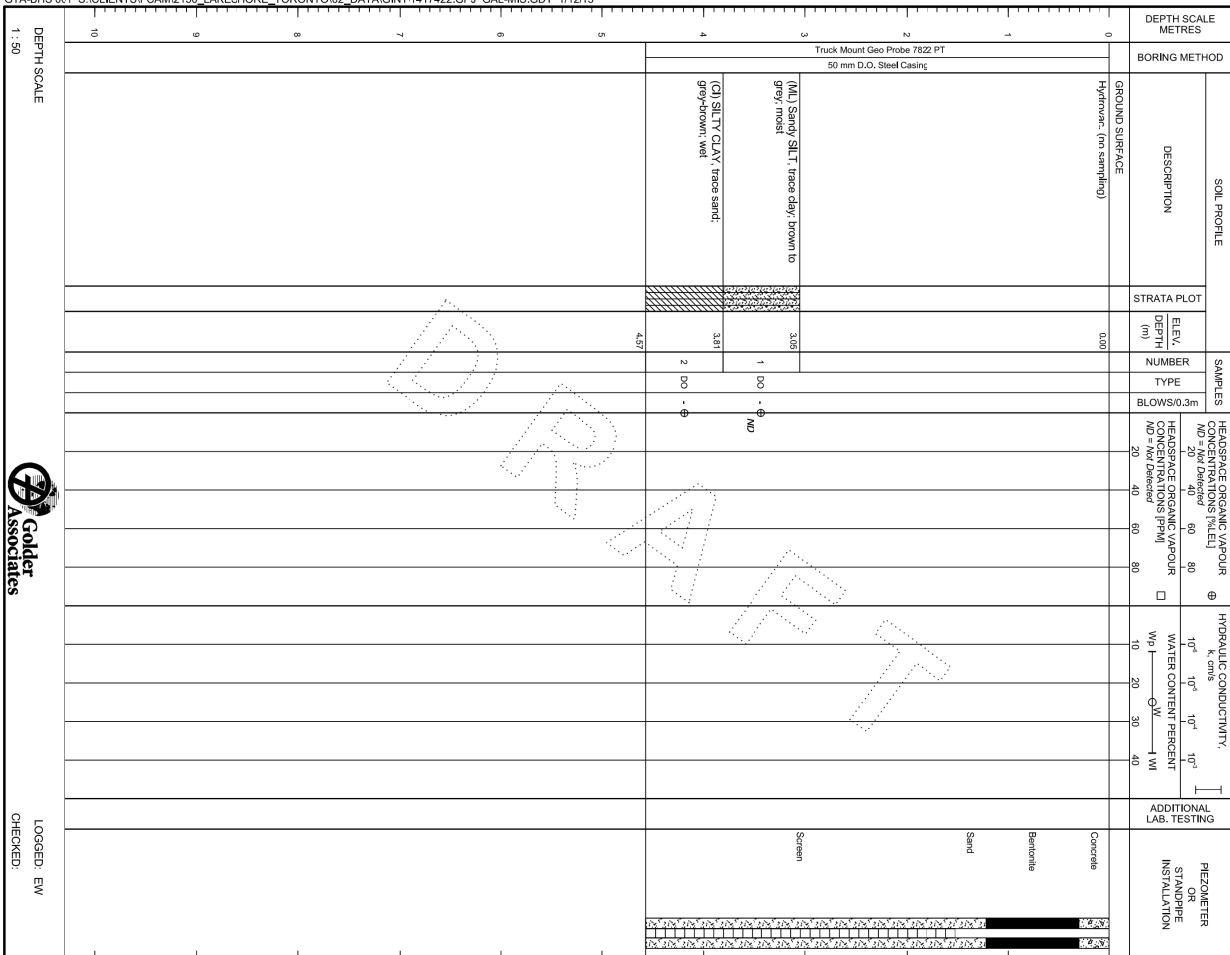
Appendix B

Relevant Borehole Logs









PROJECT: 1417422
LOCATION: N : E

RECORD OF BOREHOLE: MW14-7

BORING DATE: December 11, 2014

SHEET 1 OF 1
DATE: _____

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE	STRATA PLOT	ELEV. DEPTH (m)	SAMPLES		HEADSPACE ORGANIC VAPOUR CONCENTRATIONS (µL/L)	HYDRAULIC CONDUCTIVITY 10 ⁻⁸ 10 ⁻⁷ 10 ⁻⁶ 10 ⁻⁵	WATER CONTENT PERCENT W _p 10 20 30 40	ADDITIONAL TESTING	PIEZOMETER STANDARD INSTALLATION
					NUMBER	TYPE					
0	GROUND SURFACE	FILL - (SW) SAND, some silt, brown with grey, moist (MH) SILT, some clay, trace sand, grey, cohesive, moist		0.30	1	ND	ND = Not Detected				
1				0.61	2	ND	ND = Not Detected				
2					3	ND	ND = Not Detected				
3					4	ND	ND = Not Detected				
4					5	ND	ND = Not Detected				
5					6	ND	ND = Not Detected				
6					7	ND	ND = Not Detected				
7					8	ND	ND = Not Detected				
8					9	ND	ND = Not Detected				
9					10	ND	ND = Not Detected				
10					11	ND	ND = Not Detected				
11					12	ND	ND = Not Detected				
12					13	ND	ND = Not Detected				
13					14	ND	ND = Not Detected				
14					15	ND	ND = Not Detected				
15					16	ND	ND = Not Detected				
16					17	ND	ND = Not Detected				
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21					22	ND	ND = Not Detected				
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33					34	ND	ND = Not Detected				
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38					39	ND	ND = Not Detected				
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177					178	ND	ND = Not Detected				

REFERENCE No.: 081211

ENCLOSURE No.: 1



BOREHOLE No.: MW1-13
ELEVATION: 84.95 m

BOREHOLE REPORT

Page: 2 of 2

CLIENT: Mondelez Canada Inc.
PROJECT: Preliminary Geotechnical Investigation
LOCATION: 2150 Lake Shore Blvd. West, Toronto
DESCRIBED BY: K. Vander Meulen
DATE (START): January 28, 2013

CHECKED BY: S. Shahangian
DATE (FINISH): January 28, 2013

LEGEND

- ☐ PQ - PQ size continue coring
☒ ST - SHELBY TUBE
☐ RC - ROCK CORE
☒ WATER LEVEL

Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery	Moisture Content	Blows per 6 in. / 15 cm or ROD	Penetration Index/SCR	Shear test (Cu) Sensitivity (S) (blows / 12 in. -30 cm)	Field Lab
Feet Metres	84.95	GROUND SURFACE								
		100 mm of highly fractured rock with clay at 8.92 m depth								
				RUN-3	100	--	62	--		
		END OF BOREHOLE								
		● Borehole terminated at 10.52 m bgs ● Borehole dry to 7.3 m bgs ● 'bgs' denotes below ground surface ● Gr, Ss, Si and Cl denotes Gravel, Sand, Silt and Clay respectively								
		Ground Water Measurements : (Surface elevation : 84.95 m)								
		Date 3/4/2013								
		Depth (m) 0.68								
		Elev. (m) 84.27								

REFERENCE No.: 081211

ENCLOSURE No.: 2



BOREHOLE No.: MW2-13
ELEVATION: 81.46 m

BOREHOLE REPORT

Page: 1 of 2

CLIENT: Mondelez Canada Inc.
PROJECT: Preliminary Geotechnical Investigation
LOCATION: 2150 Lake Shore Blvd. West, Toronto
DESCRIBED BY: K. Vander Meulen
DATE (START): January 29, 2013

CHECKED BY: S. Shahangian
DATE (FINISH): January 29, 2013

LEGEND

- ☐ PQ - PQ size continue coring
☒ ST - SHELBY TUBE
☐ RC - ROCK CORE
☒ WATER LEVEL

Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery	Moisture Content	Blows per 6 in. / 15 cm or ROD	Penetration Index/SCR	Shear test (Cu) Sensitivity (S) (blows / 12 in. -30 cm)	Field Lab
Feet Metres	81.46	GROUND SURFACE								
	81.38	ASPHALT - 75 mm								
		ML-SILT, some clay, very loose, light brown, moist								
	80.55	CL-SILTY CLAY, trace sand, stiff, low plasticity, grey, moist								
		Gr : 0%, Sa : 4%, Si : 81%, Cl : 15%								
	0.91			SS-1	100	20	6-2-2-3	4		
				SS-2	62	20	3-5-6-8	11		
				SS-3	100	17	2-2-6-8	8		
				SS-4	100	17	3-5-8-8	13		
				SS-5	100	16	4-5-8-9	13		
	3.81	ML-SILT, trace clay, compact, grey, moist		SS-6	100	17	4-6-7-9	13		
				SS-7	62	15	2-4-6-6	10		
	4.88	CL-SILTY CLAY, trace sand and gravel, stiff, plastic, grey, moist		SS-8	62	24	3-1-3-2	4		
		soft, very moist to wet		SS-9	75	5	3-11-44-67	55		
	6.40	SHALE (GEORGIAN BAY FORMATION), highly to completely weathered (inferred), grey, very moist to wet		RUN-1	70	--	0	--		
		SHALE (GEORGIAN BAY FORMATION), with interbedded limestone, silstone, fissile, finely bedded to thinly laminated, grey								
		0.3 m highly fractured rock (horizontal & vertical fractures) at 6.89 m depth								
		25.4 mm clay seen at 7.16 m depth								
		50 mm vertical fracture at 7.32 m depth								
		50 mm vertical fracture with clay at 7.52 m depth								
		76 mm vertical fracture at 7.52 m depth								
		76 mm highly fractured rock with clay at 7.77 m depth		RUN-2	90	--	45	--		

BOREHOLE NO.: MW2-13
ELEVATION: 81.46 m

BOREHOLE REPORT

Page: 2 of 2

CLIENT: Mondelez Canada Inc.

PROJECT: Preliminary Geotechnical Investigation

LOCATION: 2150 Lake Shore Blvd. West, Toronto

DESCRIBED BY: K. Vander Meulen CHECKED BY: S. Shahangian

DATE (START): January 29, 2013 DATE (FINISH): January 29, 2013

LEGEND

☐ PQ - PQ size continue coring

 01 - SHELBUTUBE
 BC - ROCK CORE

 - WATER LEVEL

[illegible]

BOREHOLE NO.: MW-3-13
ELEVATION: 84.83 m

BOREHOLE REPORT

Page: 1 of 2

CLIENT: Mondelez Canada Inc

PROJECT: Preliminary Geotechnical Investigation

LOCATION: 2150 Lake Shore Blvd. West, Toronto

DESCRIBED BY: K. Vander Meulen CHECKED BY: S. Shahangian

DATE (START): January 31, 2013 DATE (FINISH): January 31, 2013

LEGEND

☐ PQ - PQ size continue coring

SI - SHELBY TUBE
BC - ROCK CORE

▲ - WATER LEVEL

Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery	Moisture Content	Blows per 6 in. / 15 cm or ROD	Penetration Index/SCR	Field	Lab
0.15	84.83	GROUND SURFACE								
0.15	84.88	TOPSOIL : 150 mm ML-SILT (FILL), trace clay, very loose to compact, brown, moist, highly oxidized		SS-1	50	20	0-0-0-4	0	0.30 m	
2.13	82.70	trace sand, blackgrey, brick fragments CL-SILT CLAY, soft, low plasticity, brown, moist		SS-3	62	18	4-4-3-2	7		
2.13	82.70			SS-4	100	20	2-1-3-4	4		
3.35	81.48	CL-ML-SILTY CLAY/CLAYEY SILT, brown, moist, stiff to very stiff		SS-5	77	17	3-6-8-12	14		
5.18	79.65	CL-SILTY CLAY, firm, plastic, grey, very moist to wet		SS-6	92	22	5-6-12-14	18		
5.18	79.65	trace clay		SS-7	72	22	10-12-13-14	25		
6.86	77.97	SHALE (GEORGIAN BAY FORMATION), highly to completely weathered (inferred), with clay inclusion, (grey)		SS-8	30	25	3-4-4-4	8		
6.86	77.97			SS-9	82	25	3-2-3-4	5		
7.40	77.43	SHALE (GEORGIAN BAY FORMATION), with interbedded limestone, fine grained siltstone, fissile, thinly-bedded to thinly laminated, grey 130 mm of highly fractured rock at 7.42 m depth 40 mm clay lens at 7.53 m depth horizontal fracture with clay infill at 7.6 m depth		SS-10	90	-	85-36-70-70 125mm	100	7.30 m	
				RUN-1	90	-	38	-		
				RUN-2	100	-	65	-		

REFERENCE No.: 081211

ENCLOSURE No.: 3

BOREHOLE No.: MW3-13
ELEVATION: 84.83 m

BOREHOLE REPORT

Page: 2 of 2

CLIENT: Mondelez Canada Inc.
PROJECT: Preliminary Geotechnical Investigation
LOCATION: 2150 Lake Shore Blvd. West, Toronto
DESCRIBED BY: K. Vander Meulen
DATE (START): January 31, 2013
CHECKED BY: S. Shanganian
DATE (FINISH): January 31, 2013

LEGEND

- ☐ PQ - PQ size continue coring
☒ ST - SHELBY TUBE
☐ RC - ROCK CORE
☒ WATER LEVEL

Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery %	Moisture Content	Blows per 6 in. / 15 cm or ROD	Penetration Index/SCR	Shear test (Cu) Sensitivity (S) (blows / 12 in. -30 cm)	Field Lab
Feet Metres	84.83	GROUND SURFACE						N	10 20 30 40 50 60 70 80 90	
		80 mm highly fractured rock with clay at 7.98 m depth								
		80 mm of fractured horizontal fracture								
		50 mm fractured rock								
		80 mm fractured rock at 10.14 m depth								
10.61 74.22		END OF BOREHOLE								
		● Borehole terminated at 10.61 m bgs								
		● Borehole dry upon completion								
		● bgs denotes below ground surface								
		Ground Water Measurements:								
		(Surface elevation : 84.83 m)								
		Date 3/4/2013								
		Depth (m) 2.90								
		Elev. (m) 81.93								

REFERENCE No.: 081211

ENCLOSURE No.: 4

BOREHOLE No.: MW4-13
ELEVATION: 84.20 m

BOREHOLE REPORT

Page: 1 of 2

CLIENT: Mondelez Canada Inc.
PROJECT: Preliminary Geotechnical Investigation
LOCATION: 2150 Lake Shore Blvd. West, Toronto
DESCRIBED BY: K. Vander Meulen
DATE (START): February 1, 2013
CHECKED BY: S. Shanganian
DATE (FINISH): February 1, 2013

LEGEND

- ☐ PQ - PQ size continue coring
☒ ST - SHELBY TUBE
☐ RC - ROCK CORE
☒ WATER LEVEL

Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery %	Moisture Content	Blows per 6 in. / 15 cm or ROD	Penetration Index/SCR	Shear test (Cu) Sensitivity (S) (blows / 12 in. -30 cm)	Field Lab
Feet Metres	84.20	GROUND SURFACE						N	10 20 30 40 50 60 70 80 90	
0.15 84.05		ASPHALT - 125 mm								
		SW-GW SAND & GRAVEL (FILL), compact, brown, moist, cobbles								
0.91 83.29		ML-SILT, trace clay, loose, dark brown, moist								
		very moist to wet, slightly dilatant, slightly oxidized								
2.29 81.91		CL-SILT CLAY, trace sand, stiff, low plasticity, brown, moist								
		Gr : 0%, Sa : 2%, SI : 69%, CI : 29%								
		SS-1		82	4	10-11-11-9	22			
		SS-2		82	22	4-4-5-4	9			
		SS-3		100	21	3-4-4-5	8			
		SS-4		82	23	5-5-7-8	12			
		SS-5		100	31	3-4-5-7	9			
		SS-6		72	10	24-15-25-30	40			
		SS-7		50	8	13-14-25/ 75mm	100			
5.18 79.02		SHALE (GEORGIAN BAY FORMATION), with interbedded limestone, siltstone fissile, thinly-bedded to thinly laminated, grey								
		100 mm section of limestone at 5.82 m depth								
		13 mm horizontal and vertical fracture								
		5.84 m depth								
		40 mm horizontal fracture with clay infill								
		13 mm clay seam at 6.33 m depth								
		25 mm highly fractured rock at 6.36 m depth								
		50 mm vertical fracture at 6.4 m depth								
		13 mm horizontal and vertical fracture at 6.89 m depth								
		13 mm highly fractured rock at 6.91 m depth								
8.23 75.97		END OF BOREHOLE								
		● Borehole terminates at 8.23 m bgs								
		● Borehole dry to 5.2 m bgs								

BOREHOLE REPORT
Page: 2 of 2

LEGEND

	PQ	- PQ size continue coring
	ST	- SHELBY TUBE
	RC	- ROCK CORE
		- WATER LEVEL


[illegible]

BOREHOLE REPORT
Page: 1 of 2


LEGEND

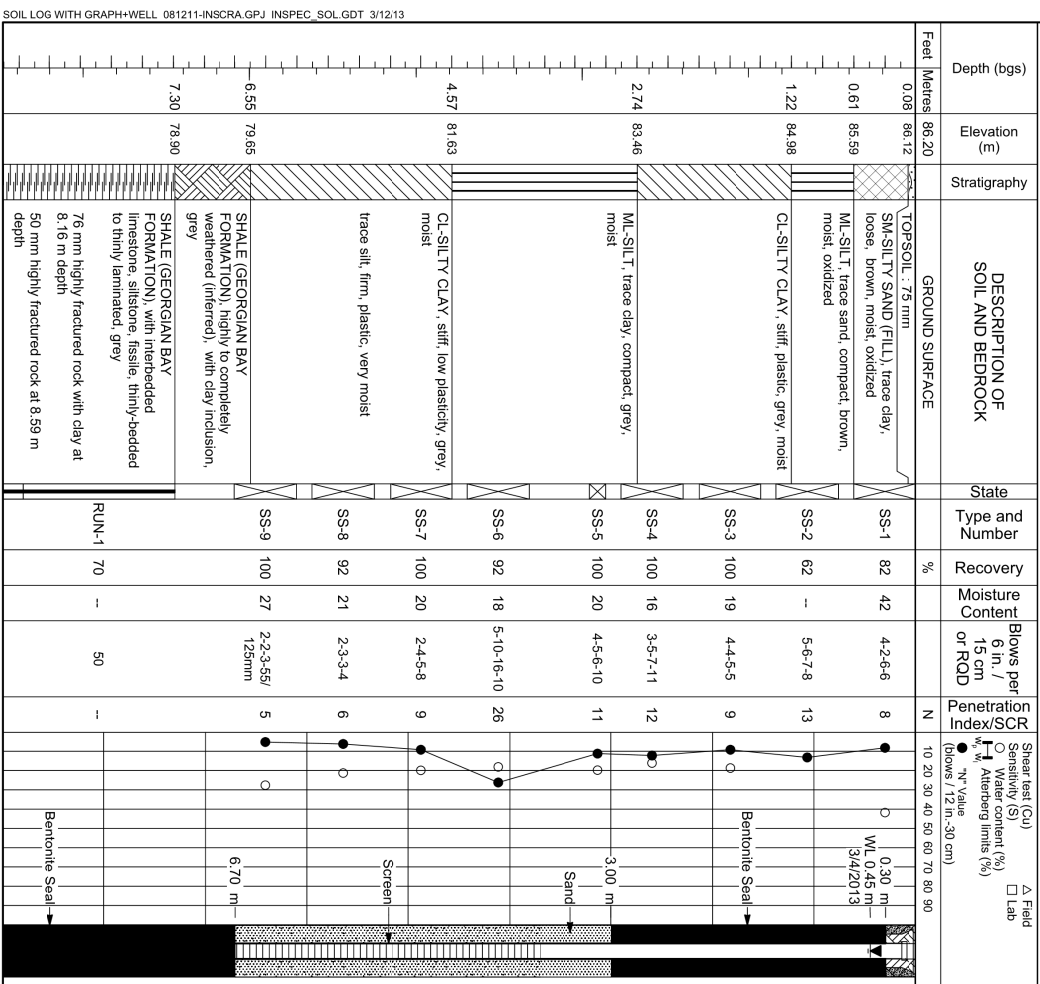
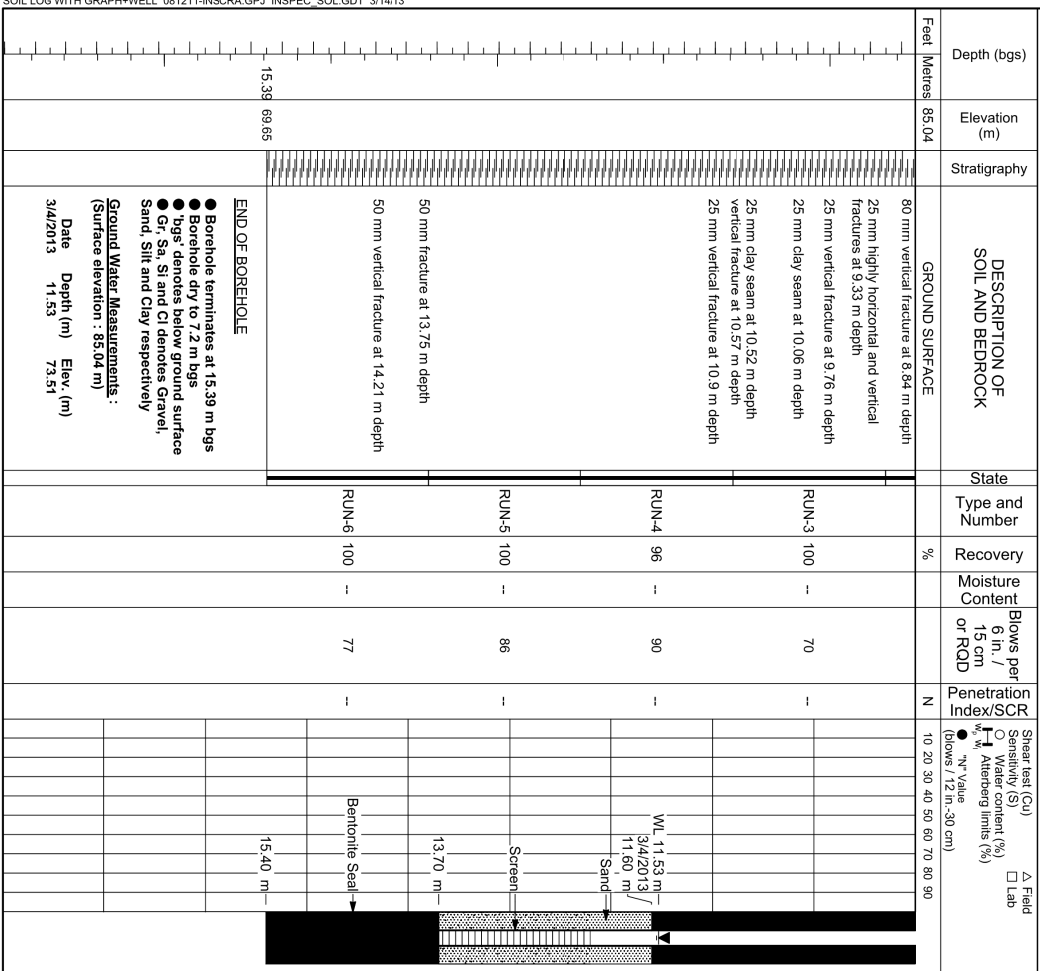
	PQ	- PQ size continue coring
	ST	- SHELBY TUBE
	RC	- ROCK CORE
		- WATER LEVEL

Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery	Moisture Content	Blows per 15 cm or RCD	Penetration Index/SCR	Shear test (Cu) Sensitivity (S) Water content (%) Atterberg limits (%) "N" Value (blows / 12 in.-30 cm)	Field Lab
0.10	85.04	GROUND SURFACE								
		TOPSOIL : 100 mm M-SILT, trace clay, loose, brown, moist, oxidized	SS-1	50	18	3-3-3-5	6			
			SS-2	50	18	3-2-3-4	5			
			SS-3	50	15	2-3-2-4	5			
		compact, highly oxidized	SS-4	62	16	10-12-15-23	27			
		grey	SS-5	100	14	8-12-13-13	25			
3.66	81.38	CL-SILT CLAY, stiff, plastic, grey, moist	SS-6	100	22	4-5-6-6	11			
		firm, plastic, very moist	SS-7	100	27	2-3-3-3	6			
		soft, very moist to wet	SS-8	100	28	2-1-2-3	3			
		Gr : 1%, Sa : 7%, Sl : 45%, Cl : 47%	SS-9	82	13	20-8-17-23/75mm	25			
6.40	78.64	SHALE (GEORGIAN BAY FORMATION), highly to completely weathered (inferred), with clay inclusion, grey								
7.21	77.83	80 mm of highly fractured rock SHALE (GEORGIAN BAY FORMATION), with interbedded limestone, siltstone, fissile, thinly-bedded to thinly laminated, grey 100 mm of highly fractured rock at 7.37 m depth 25 mm clay seam at 8.28 m depth 13 mm clay seam at 8.31 m depth 25 mm clay seam at 8.59 m depth 25 mm clay seam at 8.72 m depth	RUN-1	75	-	0	-			
			RUN-2	100	-	74	-			

CLIENT: _____ PROJECT: _____ LOCATION: _____ DESCRIBED BY: _____ DATE (START): _____	Mondelez Canada Inc. Preliminary Geotechnical Investigation 2150 Lake Shore Blvd. West, Toronto K. Vander Meulen February 4, 2013	<div style="text-align: center;">  </div> BOREHOLE No.: _____ ELEVATION: _____ MW5-13 85.04 m	BOREHOLE REPORT Page: _____ of _____
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LEGEND	
<input type="checkbox"/> PQ	- PQ size continue coring
<input checked="" type="checkbox"/> ST	- SHELBY TUBE
<input type="checkbox"/> RC	- ROCK CORE
<input checked="" type="checkbox"/> I	- WATER LEVEL

CLIENT: _____ PROJECT: _____ LOCATION: _____ DESCRIBED BY: _____ DATE (START): _____	Mondelez Canada Inc. Preliminary Geotechnical Investigation 2150 Lake Shore Blvd. West, Toronto K. Vander Meulen February 5, 2013	<div style="text-align: center;">  </div> BOREHOLE No.: _____ ELEVATION: _____ MW6-13 86.20 m
DATE (FINISH): _____ S. Shanhanganian February 5, 2013		<div style="text-align: center;"> <h2 style="margin: 0;">BOREHOLE REPORT</h2> <p style="margin: 0;">Page: <u>1</u> of <u>2</u></p> </div>





LEGEND

☐ PQ - PQ size continue coring

	SI	-SHELBI TUBE
	BC	-BACK COBE

 - WATER LEVEL

LEGEND

☐ PQ - PQ size continue coring

	SI	-SHELBI TUBE
	BC	-BACK COBE

 - WATER LEVEL

LEGEND

☐ PQ - PQ size continue coring

51 - SHELBI TUBE
BC - ROCK CORE

 - WATER LEVEL

DATE (FINISH): February 8, 20

LEGEND

☐ PQ - PQ size continue coring

51 - SHELBI TUBE
BC - ROCK CORE

 - WATER LEVEL

DATE (FINISH): February 8, 20

Feet	Metres	Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery %	Moisture Content	Blows per 15 cm or ROD	Penetration Index/SCR	Shear test (Cu) <input type="checkbox"/> Field <input type="checkbox"/> Lab Sensitivity (S) <input type="checkbox"/> Atterberg limits (%) <input type="checkbox"/> "N" Value (blows / 12 in.-30 cm)	
			86.72	GROUND SURFACE 8.72 m depth clay lens at 8.99 m depth 50 mm highly fractured rock at 10.09 m depth 25 mm horizontal and vertical fracture at 10.42 m depth END OF BOREHOLE ● Borehole terminates at 10.52 m bgs ● Borehole dry to 7.3 m depth ● bgs denotes below ground surface Ground Water Measurements : (Surface elevation : 86.72 m) Date Depth (m) Elev. (m) 3/4/2013 2.52 84.20								
						RUN-2	100	-	70	-		

Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery	Moisture Content	Blows per 6 in. / 15 cm or ROD	Penetration Index/SCR	Shear test (Cu) Sensitivity (s) Water content (%) Atterberg limits (%) "N" Value (blows / 12 in.-30 cm)	Field Lab
0.15	83.83	ASPHALT : 125 mm SW-GW SAND AND GRAVEL, (FILL), loose, brown, moist, trace cobbles		SS-1	62	5	22-6-3-3	9		
0.91	83.07	ML-SILT, trace clay, loose, grey, very moist to wet, dilatant trace sand, wet, dilatant, very loose		SS-2	100	23	2-2-3-4	5		
2.29	81.69	CL-SILT CLAY, soft, plastic, grey, very moist to wet		SS-3	62	23	2-1-2-4	3		
3.81	80.17	trace sand and gravel, soft, plastic		SS-4	100	26	2-2-1-3	3		
				SS-5	100	29	2-1-2-3	3		
				SS-6	100	27	2-1-3-4	4		
				SS-7	100	17	2-2-4-5	6		
5.34	78.64	SHALE (GEORGIAN BAY FORMATION), highly to completely weathered (interred), grey		SS-8	-	-	59/15mm	100		
5.60	78.38	SHALE (GEORGIAN BAY FORMATION), with interbedded limestone, siltstone, fissile, thinly-bedded to thinly laminated, grey		RUN-1	75	-	75	-		
				RUN-2	100	-	96	-		
				RUN-3	95	-	60	-		

WL 7.94 m
3/4/2013

8.50 m
Sand

Bentonite Seal

BOREHOLE REPORT
Page: 2 of 2

LEGEND

	PQ	- PQ size continue coring
	ST	- SHELBY TUBE
	RC	- ROCK CORE
		- WATER LEVEL

Feet	Metres	Depth (bgs)	Elevation (m)	Stratigraphy	DESCRIPTION OF SOIL AND BEDROCK	State	Type and Number	Recovery %	Moisture Content	Blows per 6 in. / 15 cm or ROD	Penetration Index/SCR	Shear test (Cu) <input type="checkbox"/> Field <input type="checkbox"/> Lab Sensitivity (S) <input type="checkbox"/> Water content (%) <input type="checkbox"/> Atterberg limits (%) <input type="checkbox"/> "N" Value <input type="checkbox"/> (Blows / 12 in. -30 cm)
			83.98		GROUND SURFACE						N	10 20 30 40 50 60 70 80 90 Screen 10.70 m— Bentonite Seal 13.90 m—
							RUN-4	100	—	95	—	
							RUN-5	100	—	100	—	
							RUN-6	100	—	100	—	
					76 mm vertical fracture at 13.62 m depth							
			13.90	70.08	END OF BOREHOLE							
					● Borehole terminates at 13.90 m bgs ● Borehole dry to 5.6 m depth ● bgs' denotes below ground surface Ground Water Measurements : (Surface elevation : 83.98 m) Date 3/4/2013 Depth (m) 7.94 Elev. (m) 76.04							

BOREHOLE REPORT
Page: 1 of 2

LEGEND

	PQ	- PQ size continue coring
	ST	- SHELBY TUBE
	RC	- ROCK CORE
		- WATER LEVEL

Feet	Metres	Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery %	Moisture Content	Blows Per 6 in. / 15 cm or ROD	Penetration Index/SCR	Shear test (Cu) Sensitivity (S) Water content (%) Atterberg limits (%) "N" Value (blows / 12 in.-30 cm)	Field <input type="checkbox"/> Lab
84.44	84.34			GROUND SURFACE						N	10 20 30 40 50 60 70 80 90	
0.10	84.34			ASPHALT : 100 mm SP-SAND (FILL), compact, brown, moist		SS-1	72	7	11-11-8-8	19	○ ●	0.30 m
1.22	83.22			ML-SILT, trace clay, compact, brown, moist some clay, loose		SS-2	62	14	4-5-7-11	12	○ ●	Bentonite Seal
2.29	82.15			CL-SILTY CLAY, soft, plastic, brown, moist		SS-3	92	21	5-3-5-6	8	○ ●	
						SS-4	62	23	3-2-2-2	4	○ ●	2.70 m WL 2.74 m 3/4/2013
3.20	81.24			ML-SILT, trace clay, compact, brown, moist some clay		SS-5	100	20	4-5-8-9	13	○ ●	Sand
4.57	79.87			CL-SILTY CLAY, stiff, low plasticity, grey, firm, plastic, very moist to wet		SS-6	100	23	3-5-8-11	13	○ ●	
						SS-7	82	22	3-4-5-5	9	○ ●	Screen
						SS-8	100	20	3-4-3-4	7	○ ●	
6.86	77.58			SHALE (GEORGIAN BAY FORMATION), highly to completely weathered (interred), grey		SS-9	92	30	2-3-2-3	5	○ ●	6.40 m
7.40	77.04			SHALE (GEORGIAN BAY FORMATION), with interbedded limestone, siltstone, fissile, thinly-bedded to thinly laminated, grey highly fractured with clay at 7.47 m depth 25 mm horizontal fracture at 7.97 m depth		SS-10	100	3	26-71-50/ 125mm	100	○ ●	
						RUN-1	100	-	80	-	-	
						RUN-2	100	-	83	-	-	Bentonite Seal

REFERENCE No.: 081211

ENCLOSURE No.: 9



BOREHOLE No.: MW9-13
ELEVATION: 84.44 m

BOREHOLE REPORT

Page: 2 of 2

CLIENT: Mondelez Canada Inc.
PROJECT: Preliminary Geotechnical Investigation
LOCATION: 2150 Lake Shore Blvd. West, Toronto
DESCRIBED BY: K. Vander Meulen
CHECKED BY: S. Shanthangan
DATE (START): January 30, 2013
DATE (FINISH): January 30, 2013

LEGEND

- ☐ PQ - PQ size continue coring
☒ ST - SHELBY TUBE
☐ RC - ROCK CORE
☒ WATER LEVEL

Depth (bgs)	Elevation (m)	Stratigraphy	State	Type and Number	Recovery %	Moisture Content	Blows per 6 in. / 15 cm or ROD	Penetration Index/SCR	Shear test (Cu) Sensitivity (S) Δ Field Δ Lab
Feet Metres 84.44		GROUND SURFACE						N	10 20 30 40 50 60 70 80 90
		13 mm clay seam at 9.65 m depth 40 mm highly fractured rock at 9.87 m depth 13 mm clay seam at 9.91 m depth 40 mm horizontal fracture with clay infill at 10.09 m depth 13 mm highly fractured rock at 10.14 m depth		RUN-3 100	--	64	--		
	10.55 73.89	END OF BOREHOLE ● Borehole terminates at 10.55 m bgs ● Borehole dry to 7.4 m depth ● 'bgs' denotes below ground surface ● Gr, Ss, Si and Ci denotes Gravel, Sand, Silt and Clay respectively Ground Water Measurements : (Surface elevation : 84.44 m) Date 3/4/2013 Depth (m) 2.74 Elev. (m) 81.70							



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO
HOLE DESIGNATION: BH2-04
DATE COMPLETED: 22 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	NUMBER	INTERVAL	PID (ppm)
	ASPHALT SAND AND GRAVEL (FILL), compact to dense, coarse grained, well graded, brown, dry	0.09			
	ML/CL - SILTY CLAY, firm to stiff, mottled gray/olive, moist	0.61	1	0.00 - 1.22	5
	- wet to saturated at 1.52m BGS				
	- thin silty sand seams (less than 1cm thick), gray, slight petroleum hydrocarbon odour at 1.83m BGS		2	1.22 - 2.44	2
	- moist to wet at 3.20m BGS				
	- gray with brown mottling, no odour at 3.35m BGS		3	2.44 - 3.35	42
	- silt, with clay, with sand, mottled brown/olive, moist to wet at 4.42m BGS		4	3.35 - 4.27	11.5
	ML - SILT (WATERLAIN), stiff, gray, moist	5.03			
	END OF TEST PIT @ 5.18m BGS	5.18	5	4.27 - 5.18	28

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS

TEST PIT LOG 32985-01.GPJ CRA CORP.GDT 21/5/04

SOIL LOG WITH GRAPH+WELL 081211-INS CRA.GPJ INSPEC SOL.GDT 3/12/13



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH3-04
DATE COMPLETED: 22 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE	
			NUMBER	PID (ppm)
0.15	ASPHALT SAND and GRAVEL (FILL), compact to dense, coarse grained, well graded, light brown, dry	0.15	1	4
0.61	M/LCL - SILTY CLAY, stiff, dark gray, dry to moist, slight petroleum hydrocarbon odour	0.61	2	148
1.22	- soft, wet at 1.22m BGS	1.22	3	78
2.13	- moderate to strong petroleum hydrocarbon odour at 2.13m BGS	2.13	4	2
2.44	- thin sand silt seams (less than 1cm thick), moderate to strong petroleum hydrocarbon odour at 2.44m BGS	2.44	5	1
2.74	- slight odour at 2.74m BGS	2.74		
3.05	- no odour at 3.05m BGS	3.05		
4.57	- soft, very moist to wet at 4.57m BGS	4.57		
5.18	END OF TEST PIT @ 5.18m BGS	5.18		

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE
CHEMICAL ANALYSIS



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH4-04
DATE COMPLETED: 22 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE	
			NUMBER	PID (ppm)
0.15	ASPHALT SAND and GRAVEL (FILL), compact to dense, coarse grained, well graded, light brown, dry	0.15	1	7
0.46	- with silt, trace gravel at 0.46m BGS	0.46	2	19
0.76	M/LCL - SILTY CLAY, stiff, dark gray, moist, black staining, petroleum hydrocarbon odour	0.76	3	334
2.74	- trace sand, moderate to slight odour at 2.74m BGS	2.74	4	4
3.20	- stiff, moist, no odour at 3.20m BGS	3.20		
3.81	- soft to firm, very moist to wet at 3.81m BGS	3.81		
4.11	- stiff, moist at 4.11m BGS	4.11		
4.27	END OF TEST PIT @ 4.27m BGS	4.27		

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE
CHEMICAL ANALYSIS



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH6-04
DATE COMPLETED: 22 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE		
			NUMBER	INTERVAL	PID (ppm)
0.15	ASPHALT SAND and GRAVEL (FILL), compact to dense, coarse grained, well graded, light brown, dry	0.15	1	0.00 - 1.22	1
0.5	- sand, with silt, compact, poorly graded, brown, moist at 0.61m BGS				
1.0	MLCL - SILTY CLAY, stiff, mottled brown/gray, moist	1.22	2	1.22 - 2.44	34
1.5	- wet at 1.52m BGS				
2.0	- gray, some brown mottling, moderate petroleum hydrocarbon odour at 1.98m BGS				
2.5	- gray, saturated, strong petroleum hydrocarbon odour at 2.44m BGS		3	2.44 - 3.35	125
3.0					
3.5	- slight odour at 3.35m BGS				
4.0	- no odour at 3.81m BGS		4	3.35 - 4.27	2
4.5	END OF TEST PIT @ 4.27m BGS	4.27			
5.0					
5.5					
6.0					
6.5					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE
CHEMICAL ANALYSIS



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH6-04
DATE COMPLETED: 22 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE		
			NUMBER	INTERVAL	PID (ppm)
0.46	SAND and GRAVEL (FILL), compact to dense, coarse grained, brown, dry	0.46	1	0.00 - 1.22	1.4
0.5	SAND (FILL), loose, fine to medium grained, massive, light brown, dry, no odour				
1.0	MLCL - SILTY CLAY, stiff, low plasticity, massive, gray with black mottling, dry to moist	1.01	2	1.22 - 2.44	13.5
1.5	- slight to moderate petroleum hydrocarbon odour at 1.22m BGS				
2.0	- trace sand, soft, gray, saturated, strong petroleum hydrocarbon odour at 1.52m BGS				
2.5			3	2.44 - 3.35	-
3.0					
3.5	- moist, slight petroleum hydrocarbon odour at 3.35m BGS				
4.0	- stiff, mottled gray/brown, no odour at 3.65m BGS		4	3.35 - 4.27	2.5
4.5	END OF TEST PIT @ 4.27m BGS	4.27			
5.0					
5.5					
6.0					
6.5					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE
CHEMICAL ANALYSIS



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH7-04
DATE COMPLETED: 22 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE		
			NUMBER	INTERVAL	PID (ppm)
	SAND and GRAVEL (FILL), compact, coarse grained, well graded, light brown/gray, dry		1	0.00 - 1.22	0
	SAND (FILL), loose, medium grained, poorly graded, brown, dry to moist	0.76			
	- silty sand, dense, dilatant, saturated at 2.44m BGS - black petroleum hydrocarbon staining, strong petroleum hydrocarbon odour at 2.74m BGS		2	1.22 - 2.44	0
			3	2.44 - 3.66	15.5
	SANDY SILT, stiff, brown, moist, no odour	4.27			
	END OF TEST PIT @ 4.57m BGS	4.72	4	3.66 - 4.72	0

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH8-04
DATE COMPLETED: 22 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE		
			NUMBER	INTERVAL	PID (ppm)
	ASPHALT/CONCRETE				
	SILTY CLAY (FILL), with gravel, stiff, mottled gray/brown, dry to moist	0.21	1	0.00 - 1.22	0
	MLCL - SILTY CLAY, stiff, olive, moist, no odour	1.07			
	- soft to firm, dilatant, brown, very moist, no odour at 1.52m BGS - stiff, moist at 2.74m BGS		2	1.22 - 2.44	0
			3	2.44 - 3.35	0
	END OF TEST PIT @ 4.27m BGS	4.27	4	3.35 - 4.27	0

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH9-04
DATE COMPLETED: 23 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE	
			NUMBER	PID (ppm)
	ASPHALT	0.09		
	SAND and GRAVEL (FILL), compact to dense, coarse grained, well graded, light brown, dry			
	- with silt at 0.30m BGS			
0.5	MLCL - SILTY CLAY, firm to stiff, dark gray, moist, no odour	0.46	1	0
			INTERVAL	
			0.00 - 1.22	
1.0				
1.5	- soft, brown mottling, very moist to wet at 1.83m BGS		2	0
			INTERVAL	
			1.22 - 2.44	
2.0	- sandy seam (10cm thick), light gray at 2.13m BGS			
2.5	- saturated at 2.44m BGS		3	0
			INTERVAL	
			2.44 - 3.35	
3.0				
3.5	- small sand seams (less than 1cm thick), dark gray/black, no odour at 3.35m BGS		4	0
			INTERVAL	
			3.35 - 4.27	
4.0				
4.5	- mottled gray/brown at 4.27m BGS		5	0
			INTERVAL	
			4.27 - 5.33	
5.0				
5.5	END OF TEST PIT @ 5.33m BGS	5.33		
6.0				
6.5				

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

TEST PIT LOG 32985-01.GPJ CRA CORP.GDT 21/5/04



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH10-04
DATE COMPLETED: 23 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE	
			NUMBER	PID (ppm)
	ASPHALT	0.09		
	SAND and GRAVEL (FILL), compact to dense, coarse grained, well graded, light brown, dry			
0.5	SAND (FILL), compact, medium grained, poorly graded, brown, moist	0.46	1	0
			INTERVAL	
			0.00 - 1.22	
1.0				
1.5	MLCL - SILTY CLAY, stiff, brown with gray mottling, moist	1.52	2	0
			INTERVAL	
			1.22 - 2.44	
2.0				
2.5	- thin gray sand seams (less than 1cm thick), moderate petroleum hydrocarbon odour at 2.59m BGS		3	330
			INTERVAL	
			2.44 - 3.35	
3.0				
3.5	- stiff, mottled brown/gray, no odour at 3.51m BGS		4	30
			INTERVAL	
			3.35 - 4.27	
4.0				
4.5	- soft, silty, saturated at 4.27m BGS		5	9
	- gray at 4.42m BGS		INTERVAL	
			4.27 - 5.18	
5.0				
5.5	END OF TEST PIT @ 5.18m BGS	5.18		
6.0				
6.5				

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS

TEST PIT LOG 32985-01.GPJ CRA CORP.GDT 21/5/04



STRATIGRAPHIC LOG

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH11-04
DATE COMPLETED: 23 January 2004
DRILLING METHOD: DIRECT-PUSH
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE	
			NUMBER	PID (ppm)
	CONCRETE	0.15		
	SANDY SILT (FILL), stiff, brown, moist		1	4
			INTERVAL	
			0.00 - 1.22	
	- saturated at 1.37m BGS			
	- black staining, strong petroleum hydrocarbon odour at 1.68m BGS		2	670
			INTERVAL	
			1.22 - 2.44	
	- no staining at 2.29m BGS			
	- moist to dry, strong petroleum hydrocarbon odour at 2.59m BGS		3	150
			INTERVAL	
			2.44 - 3.35	
	- moderate odour at 3.05m BGS			
	- sand seam (approximately 10cm thick), saturated at 3.35m BGS		4	120
			INTERVAL	
			3.35 - 4.27	
	ML - SILT (WATERLAIN), with clay to clayey, with sand, soft, high plasticity, gray, very moist	4.11		
			INTERVAL	
			4.27 - 5.18	
	- trace clay, low plasticity, saturated at 5.18m BGS		5	60
			INTERVAL	
			5.18 - 6.10	
	- soft, high plasticity, dilatant, very moist at 6.10m BGS		6	25
			INTERVAL	
			6.10 - 6.86	
	- shale fragments in spoon tip at 6.71m BGS		7	13
			INTERVAL	
			6.86 - 6.86	
	END OF TEST PIT @ 6.86m BGS	6.86		

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: MM1-04
DATE COMPLETED: January 20, 2004
DRILLING METHOD: 4 1/4" ID HSA
FIELD PERSONNEL: B. IOTZOV

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE			
			NUMBER	INTERVAL	REC (%)	PID (ppm)
	ASPHALT	0.46				
	SILT and CLAY (FILL), soft, gray, wet to saturated, no odour		1	17	8	0
			INTERVAL			
				33	4	0
			2	42	1	0
			INTERVAL			
				8	3	0
	SILT (FILL), with clay to clayey, trace sand, soft, high plasticity, laminated, gray, saturated	3.96				
			INTERVAL			
				58	3	0
			5	20	0	
			INTERVAL			
				50	20	0
	SHALE (BEDROCK), gray	5.33				
	END OF BOREHOLE @ 5.68m BGS	5.49				
			INTERVAL			
				0	>50	-
			7			

WELL DETAILS

Screened interval: 1.52 to 4.57m BGS

Length: 3.05m

Diameter: 51mm

Sid Size: 10

Material: SCH 40 PVC

Seal: 0.30 to 1.22m BGS

Material: BENTONITE CHIPS

Sand Pack: 1.22 to 4.72m BGS

Material: #1 SILICA SAND

MONITOR INSTALLATION

CONCRETE SEAL

BENTONITE CHIPS

203mm Ø BOREHOLE

WELL SCREEN SAND PACK

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: MW2-04
DATE COMPLETED: January 21, 2004
DRILLING METHOD: 4 1/4" ID HSA
FIELD PERSONNEL: B. IOTZOV

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	MONITOR INSTALLATION	SAMPLE			
				NUMBER	INTERVAL	REC (%)	'N' VALUE
0.5	ASPHALT	0.46	CONCRETE SEAL	1	75	16	0
0.5	SAND (FILL), dense, brown, moist - silt, with sand, laminated, mottled gray/green at 0.61m BGS	0.76	BENTONITE CHIPS	2	60	18	0
1.0	SILT, trace sand, trace gravel, firm, laminated, mottled gray/green, moist			3	8	3	0
1.5				4	65	4	0
2.0	- wet at 2.29m BGS			5	65	23	0
2.5				6	12	0	
3.0	- moist at 3.36m BGS			7	>62	0	
3.5							
4.0							
4.5	- with gravel at 4.57m BGS						
5.0	END OF BOREHOLE @ 4.97m BGS	4.97					

WELL DETAILS:
Screened interval: 1.52 to 4.57m BGS
Length: 3.05m
Diameter: 51mm
Silt Size: 10
Material: SCH 40 PVC
Seal: 0.30 to 1.22m BGS
Material: BENTONITE CHIPS
Sand Pack: 1.22 to 4.57m BGS
Material: #1 SILICA SAND

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: MW3-04
DATE COMPLETED: January 21, 2004
DRILLING METHOD: 4 1/4" ID HSA
FIELD PERSONNEL: B. IOTZOV

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	MONITOR INSTALLATION	SAMPLE			
				NUMBER	INTERVAL	REC (%)	'N' VALUE
0.5	ASPHALT	0.30	CONCRETE SEAL	1	56	41	0
0.5	SAND and GRAVEL (FILL), dense, gray/brown, slight petroleum hydrocarbon odour		BENTONITE CHIPS	2	0	13	-
1.0	SAND and SILT (FILL), firm, gray	0.91		3	50	2	0
1.5	- saturated at 1.22m BGS			4	38	4	0
2.0				5	100	4	0
2.5				6	88	9	0
3.0	- (trace gravel, trace sand, trace clay at 3.66m BGS			7	71	17	0
3.5							
4.0							
4.5	ML - SILT (FILL), trace sand, trace gravel, stiff medium plasticity, laminated, gray/olive, moist to wet	4.27					
5.0	- with sand, with gravel at 4.88m BGS						
5.5	END OF BOREHOLE @ 5.18m BGS	5.18					

WELL DETAILS:
Screened interval: 1.52 to 4.57m BGS
Length: 3.05m
Diameter: 51mm
Silt Size: 10
Material: SCH 40 PVC
Seal: 0.30 to 1.22m BGS
Material: BENTONITE CHIPS
Sand Pack: 1.22 to 4.72m BGS
Material: #1 SILICA SAND

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: MW5-04
DATE COMPLETED: December 22, 2004
DRILLING METHOD: DIRECT-PUSH / 4 1/4" ID HSA
FIELD PERSONNEL: N. REDWOOD, J. ONEIL

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	MONITORING WELL	SAMPLE			
				NUMBER	INTERVAL	REC (%)	PID (ppm)
0.20	ASPHALT	0.20		1	P/S	75	0.6
0.36	GRAVEL (FILL) with sand, trace silt, loose, fine to coarse grained, well graded, brownish grey, moist SAND (FILL) trace silt, compact, fine to medium grained, poorly graded, brown with oxidation, moist to wet, no odours - grey with dark grey mottling, wet, no odours at 0.76m BGS	0.36		2	P/S	75	3.2 0.6
0.97	ML - SILT, with trace clay, trace sand, firm to stiff, slightly to non-plastic, grey with green mottling, vegetative debris, moist - with sand, fine grained, trace oxidation, no vegetative debris at 1.22m BGS - trace sand, coarse grained, grey, oxidation mottling, moist to wet, sheen, strong petroleum hydrocarbon odour, black grease from 1.52 to 1.83m BGS	0.97		3	P/S	75	1.4
3.20	- black vegetative debris at 3.05m BGS SHALE - SANDY SILT, slightly cohesive, fine grained, non plastic, slightly dilatant, grey, moist to wet, - not dilatant, moist at 3.51m BGS	3.20		4	P/S	75	1.0
4.57	ML - SILT (FILL) with clay, trace gravel, trace sand, hard, well graded, fine to coarse grained, cohesive, plastic, grey, moist END OF BOREHOLE @ 4.57m BGS	4.57					
4.88		4.88					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: UST CLOSURE SERVICES
PROJECT NUMBER: 32985-01
CLIENT: KRAFT CANADA INC.
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: MW6-04
DATE COMPLETED: December 22, 2004
DRILLING METHOD: DIRECT-PUSH / 4 1/4" ID HSA
FIELD PERSONNEL: N. REDWOOD, J. ONEIL


DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	MONITORING WELL	SAMPLE			
				NUMBER	INTERVAL	REC (%)	PID (ppm)
0.15	ASPHALT	0.15		1	P/S	50	0.8
0.20	GRAVEL (FILL), trace to with sand, trace silt, loose, well graded, small to coarse grained, grey, moist SAND (FILL), with gravel, trace silt, fine to coarse grained, brown, moist - no gravel, loose, fine grained, wet, oxidized at 0.61m BGS	0.20		2	P/S	75	0.2
2.29	- grey, black mottling at 1.07m BGS - trace to with silt, dilatant at 1.37m BGS	2.29		3	P/S	75	0.2
4.72	- dark grey, slight staining, slight odours at 2.13m BGS ML - SILT, trace to with clay, trace sand, stiff, fine grained, slightly cohesive, brittle, grey with green mottling, moist, oxidized - clayey, plastic, cohesive, sticky, moist to wet at 2.74m BGS	4.72		4	P/S	75	1.3
5.18	- medium to coarse sand veneer, brown, wet at 3.25m BGS - trace fine sand veneers, grey at 3.51m BGS	5.18		5	P/S	75	1.1
5.64	- grey, no mottling, moist at 3.96m BGS ML - SILT (FILL) with clay, trace to with sand, trace gravel, stiff, fine to coarse grained, cohesive, plastic, grey, moist - trace clay, sticky plastic, very cohesive, moist to wet at 4.61m BGS SHALE - bedrock, grey, brittle, moist END OF BOREHOLE @ 5.64m BGS	5.64					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS

PROJECT NAME: UST CLOSURE SERVICES	HOLE DESIGNATION: BH202-05
PROJECT NUMBER: 32986-02	DATE COMPLETED: November 14, 2005
CLIENT: KRAFT CANADA INC.	DILLING METHOD: 2" PERCUSSION/BOSSCH
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO	FIELD PERSONNEL: K. PETER

OVERBURDEN LOG 32385-02 GPJ CRA_CORP GDT 1/6/06


DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE				
			NUMBER	INTERVAL	REC (%)	'N' VALUE	Eagle (ppm)
	CONCRETE FLOOR SLAB	0.15					
	GRANULAR FILL	0.38					
	ML/SML - SILTY SANDS/SANDY SILT, compact/firm, fine grained, dark grey, moist, mild petroleum hydrocarbon odour - dark grey/black staining, stronger petroleum hydrocarbon odour from 0.61 to 0.76m BGS - strong petroleum hydrocarbon odour, some sheen on soil surface at 1.37m BGS - well-saturated, dilatant, moderate petroleum hydrocarbon odour, fainter with depth at 1.52m BGS - seams of silty clay, grey, stiff, moist from 1.52 to 2.59m BGS						
2.5	END OF BOREHOLE @ 2.59m BGS	2.59					
3.0							
3.5							
4.0							
4.5							
5.0							
5.5							
6.0							
6.5							

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS

PROJECT NAME: UST CLOSURE SERVICES	HOLE DESIGNATION: BH202-05
PROJECT NUMBER: 32986-02	DATE COMPLETED: November 14, 2005
CLIENT: KRAFT CANADA INC.	DILLING METHOD: 2" PERCUSSION/BOSSCH
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO	FIELD PERSONNEL: K. PETER

OVERBURDEN LOG 32385-02 GPJ CRA_CORP GDT 1/6/06

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE				
			NUMBER	INTERVAL	REC (%)	'N' VALUE	Eagle (ppm)
	CONCRETE FLOOR SLAB	0.15					
	GRANULAR FILL	0.38					
	M/L/S/M - SILTY SANDS/SANDY SILT, compact/firm, fine grained, dark grey, moist, mild petroleum hydrocarbon odour - dark grey/black staining, stronger petroleum hydrocarbon odour from 0.61 to 0.76m BGS - strong petroleum hydrocarbon odour, some sheen on soil surface at 1.37m BGS - well-saturated, dilatant, moderate petroleum hydrocarbon odour, fainter with depth at 1.52m BGS - seams of silty clay, grey, stiff, moist from 1.52 to 2.59m BGS						
2.5	END OF BOREHOLE @ 2.59m BGS	2.59					
3.0	Note: Borehole was backfilled with bentonite chips and a concrete surface. Borehole was refilled from 1.5 to 2.3 m bgs.						
3.5							
4.0							
4.5							
5.0							
5.5							
6.0							
6.5							

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



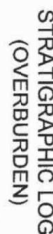
PROJECT NAME: UST CLOSURE SERVICES	HOLE DESIGNATION: BH203-05
PROJECT NUMBER: 32956-02	DATE COMPLETED: November 14, 2005
CLIENT: KRAFT CANADA INC.	DILLING METHOD: 2" PERCUSSION/BOSCH
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO	FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE			
			NUMBER	INTERVAL	REC (%)	'N' VALUE
	CONCRETE FLOOR SLAB	0.15				
	GRANULAR FILL, no staining or odour	0.46				
0.5	SM - SAND with silt, compact, fine to medium grained, brown, moist, some black coal clasts - silty, dark grey, wet/saturated, dilutant, faint petroleum hydrocarbon odour at 0.76m BGS		SS-1	15		0
1.0			SS-2	100		0
1.5	END OF BOREHOLE @ 1.52m BGS Note: Borehole was backfilled with bentonite chips and a concrete surface.	1.52				
2.0						
2.5						
3.0						
3.5						
4.0						
4.5						
5.0						
5.5						
6.0						
6.5						

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS ○

OVERBUREAU LOG 32985-02 GP2 CRA CORP.GD1 1606



PROJECT NAME: UST CLOSURE SERVICES	HOLE DESIGNATION: BH204-05
PROJECT NUMBER: 32985-02	DATE COMPLETED: November 14, 2005
CLIENT: KRAFT CANADA INC.	DILLING METHOD: 2" PERCUSSION/BOSCH
LOCATION: 2150 LAKESHORE BLVD. W., TORONTO, ONTARIO	FIELD PERSONNEL: K. PETER

VERBURDEN LOG 32985-02 GPJ CRA CORP GDT6/8/06


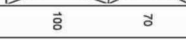
DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE				
			NUMBER	INTERVAL	REC (%)	'N' VALUE	Eagle (ppm)
	CONCRETE FLOOR SLAB - no recovery, very soft, spoons appear saturated from 0.20 to 0.76m BGS	0.20					
	ML/SM - SILTY SAND, soft, fine grained, brown, saturated, dilatant - dark grey, faint petroleum hydrocarbon odour, some staining/streak on soil at 1.37m BGS		SS-1	0			5
			SS-2	20			5
			SS-2	20			5
	CL/ML - CLAYEY SILT, stiff, grey, moist, no odour or staining		SS-3	100			5
	END OF BOREHOLE @ 2.29m BGS Note: Borehole was backfilled with bentonite chips and a concrete surface.	2.29					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE, REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



HOLE DESIGNATION: BH205-05
DATE COMPLETED: November 14, 20
DRILLING METHOD: 2" PERCUSSION
FIELD PERSONNEL: K. PETER

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	SAMPLE				
			NUMBER	INTERVAL	REC (%)	'N' VALUE	Eagle (ppm)
	CONCRETE FLOOR SLAB SMML - SILTY SAND, compact to dense, fine grained, brown, moist, no odour or staining	0.15			70		0
	- saturated, grey, trace clay at 1.07m BGS				100		0
	END OF BOREHOLE @ 1.52m BGS	1.52					
	Note: Borehole was backfilled with bentonite chips and a concrete surface.						
					</		



DRILLING DATA
 Method: Geo Probe
 Diameter:
 Date: Nov/06/2013

REF. NO.: 1889-220
ENCL NO.:

ENCL NO.:


[illegible]

GRAPH NOTES

$+3 \times 3$	Numbers refer to Sensitivity
$\circ \epsilon = 3\%$	Strain at Failure

PROJECT: Mr. Chishe
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/6/2013
REF. NO.: 1889-220
ENCL. NO.:

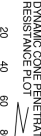
SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (T) 					PLASTIC MOISTURE LIMIT		NATURAL MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER			20	40	60	80	100	w _p	w _L	w	w _L			
0.0	CONCRETE 230 mm of reinforced concrete		1AUNDIST														
0.2	FILL sand and silt, grey/brown, moist																
0.6	CLAYEY SILT trace sand, grey/brown, moist		1BUNDIST														
1.2	SILTY CLAY trace sand, trace gravel, grey, wet		2AUNDIST														
			2BUNDIST														
			3AUNDIST														
			3BUNDIST														
			4AUNDIST														
			4BUNDIST														
5.5	END OF BOREHOLE 1. Sample refusal at 5.5 m. 2. Borehole backfilled with bentonite upon completion.		5AUNDIST														

GROUNDWATER ELEVATIONS
Shallow Single Installation  Deep Dual Installation 

GRAPH NOTES + 3, x 3: Numbers refer to Sensitivity  Span at Failure

PROJECT: Mr. Chishe
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/05/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (T) 					PLASTIC MOISTURE LIMIT		NATURAL MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER			20	40	60	80	100	w _p	w _L	w	w _L			
0.0	CONCRETE 150 mm of reinforced concrete		1AUNDIST														
0.2	FILL sand, trace gravel, trace silt, brown, moist																
0.9	SILTY CLAY trace sand, trace gravel, brown, moist		1BUNDIST														
1.2	SILT some clay, trace sand, trace gravel, brown, moist		2AUNDIST														
			2BUNDIST														
2.4	SILTY CLAY brown, moist		3AUNDIST														
			3BUNDIST														
			4AUNDIST														
4.9	END OF BOREHOLE 1. Sample refusal at 4.9 m. 2. Borehole backfilled with bentonite upon completion		4BUNDIST														

GROUNDWATER ELEVATIONS
Shallow Single Installation  Deep Dual Installation 

GRAPH NOTES + 3, x 3: Numbers refer to Sensitivity  Span at Failure

PROJECT: Mr. Christie
CLIENT: [REDACTED]
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/6/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (TLOT)	PLASTIC MOISTURE LIMIT W _p W _L	NATURAL MOISTURE W _n	POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER								
0.0	CONCRETE										
0.2	230 mm of reinforced concrete sand and silt, brown/grey/moist FILL		1AUNDIST								
			1BUNDIST								
			1CUNDIST								
			2AUNDIST								
			2BUNDIST								
			2CUNDIST								
			3AUNDIST								
			3BUNDIST								
			3CUNDIST								
			4AUNDIST								
			4BUNDIST								
			4CUNDIST								
			5AUNDIST								
1.8	SILTY CLAY grey, saturated		2BUNDIST								
			2CUNDIST								
			3AUNDIST								
			3BUNDIST								
			3CUNDIST								
			4AUNDIST								
			4BUNDIST								
			4CUNDIST								
			5AUNDIST								
5.5	END OF BOREHOLE 1. Sample refusal at 5.5 m. 2. Borehole backfilled with bentonite upon completion.										

GROUNDWATER ELEVATIONS

Shallow Single Installation  Deep Dual Installation 

GRAPH NOTES

+ 3' x 3' : Numbers refer to Sensitivity

○ 6-3% Span at Failure

PROJECT: Mr. Christie
CLIENT: [REDACTED]
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/07/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (TLOT)	PLASTIC MOISTURE LIMIT W _p W _L	NATURAL MOISTURE W _n	POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER								
0.0	CONCRETE										
0.2	150 mm of reinforced concrete sand, trace silt, brown, moist FILL		1AUNDIST								
0.6	SILTY CLAY trace sand, brown, moist		1BUNDIST								
			1CUNDIST								
			2AUNDIST								
			2BUNDIST								
			2CUNDIST								
			3AUNDIST								
			3BUNDIST								
			3CUNDIST								
			4AUNDIST								
			4BUNDIST								
			4CUNDIST								
			5AUNDIST								
			5BUNDIST								
6.1	END OF BOREHOLE 1. Sample refusal at 6.6 m. 2. Borehole backfilled with bentonite upon completion.										

GROUNDWATER ELEVATIONS

Shallow Single Installation  Deep Dual Installation 

GRAPH NOTES

+ 3' x 3' : Numbers refer to Sensitivity

○ 6-3% Span at Failure

PROJECT: Mr. Chrislie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/07/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DEPTH	DESCRIPTION	STRATA PLOT			20	40	60	80	100	W _p	W _L			
0.0		CONCRETE													
0.2		FILL 150 mm of reinforced concrete sand, trace silt, gravel, brown, moist	1AUNDIST												
			1BUNDIST												
1.4		SILT trace clay, grey, moist	2AUNDIST												
1.8		CLAYEY SILT grey, moist	2BUNDIST												
			2BUNDIST												
			3AUNDIST												
			3BUNDIST												
3.7		SILTY CLAY grey, moist	4AUNDIST												
			4AUNDIST												
			4BUNDIST												
			5AUNDIST												
			5BUNDIST												
6.7		END OF BOREHOLE 1. Sample refusal at 6.7 m. 2. Borehole backfilled with bentonite upon completion.	6AUNDIST												

GROUNDWATER ELEVATIONS
Shallow Single Installation: [Symbol] Deep Dual Installation: [Symbol]
GRAPH NOTES + 3, - 3: Numbers refer to Sensitivity ○ 5-3% Span at Failure

PROJECT: Mr. Chrislie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/06/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DEPTH	DESCRIPTION	STRATA PLOT			20	40	60	80	100	W _p	W _L			
0.0		CONCRETE													
0.2		FILL 150 mm of reinforced concrete sand and clayey silt, brown/grey	1AUNDIST												
			1BUNDIST												
1.1		SILT some clay, brown, moist	2AUNDIST												
			2AUNDIST												
			2BUNDIST												
2.4		SILTY CLAY grey, wet	3AUNDIST												
			3AUNDIST												
			3BUNDIST												
			4AUNDIST												
			4BUNDIST												
			5AUNDIST												
			5BUNDIST												
6.2		END OF BOREHOLE 1. Sample refusal at 6.2 m. 2. Borehole backfilled with sand to 5.8 m. 3. 300mm-diameter monitoring well installed. 4. Water encountered at 3.4 mbg on Nov 6, 2013.	6AUNDIST												

GROUNDWATER ELEVATIONS
Shallow Single Installation: [Symbol] Deep Dual Installation: [Symbol]
GRAPH NOTES + 3, - 3: Numbers refer to Sensitivity ○ 5-3% Span at Failure

PROJECT: Mr Christie
CLIENT: [REDACTED]
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/06/2013




REF. NO.: 1889-220
ENCL NO.:

[illegible]

PROJECT: Mr. Christie
CLIENT: [REDACTED]
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:



DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/05/2013


REF. NO.: 1889-220
ENCL NO.:

SOIL PROFILE		SAMPLES			DYNAMIC CONE PENETRATION PRESSURE (TS) 	PLASTIC LIMIT MOISTURE CONTENT  NATURAL LIQUID LIMIT  WATER CONTENT (%)	POCKET PEN. (Cu) (MPa)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
(m) ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER TYPE "N" BLOWS 0.3 m	ELEVATION				
0.0	CONCRETE		1AUNDIST					
0.2	FILL 150 mm of reinforced-concrete sand, trace gravel, trace silt, brown, moist							
	wet		2AUNDIST					
	grey		2BUNDIST					
2.4	END OF BOREHOLE 1. Sample refusal at 2.4 m. 2. 30mm-diameter monitoring well installed 3. No water encountered at 2.1 m bty Nov. 5, 2013							

[illegible]

GROUNDWATER ELEVATIONS

Shallow Single Installation  

Deep Dual Installation  

GRAPH NOTES

$+3 \times 3$: Numbers refer to Sensitivity

$\phi = 3\%$ Strain at Failure

[illegible]

GROUNDWATER ELEVATIONS

Shallow Single Installation Deep Dual Installation

GRAPH NOTES

$+3, \times 3$: Numbers refer to Sensitivity

☐ $\pm 3\%$ Strain at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/6/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER		RESISTANCE PLOT					W _p	W _L			
			TYPE		20	40	60	80	100	NATURAL MOISTURE				
					SHEAR STRENGTH (kPa)					WATER CONTENT (%)				
					● QUICK TRIAXIAL × LAB WANE									
0.0	CONCRETE													
0.2	200 mm of reinforced concrete													
	FILL													
	silt, some clay, grey, moist		1AUNDIST											
			1BUNDIST											
	grey/brown		2AUNDIST											
			2BUNDIST											
			3AUNDIST											
3.0	SILTY CLAY													
	grey, wet		3BUNDIST											
			4AUNDIST											
			4BUNDIST											
			5AUNDIST											
	trace gravel		5BUNDIST											
			6AUNDIST											
6.2	END OF BOREHOLE													
	1. Sample refusal at 6.2 m. 2. Borehole backfilled with bentonite upon completion.													

GROUNDWATER ELEVATIONS

Shallow Single Installation  Deep Dual Installation 

GRAPH NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/07/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER		RESISTANCE PLOT					W _p	W _L			
			TYPE		20	40	60	80	100	NATURAL MOISTURE				
					SHEAR STRENGTH (kPa)					WATER CONTENT (%)				
					● QUICK TRIAXIAL × LAB WANE									
0.0	CONCRETE													
0.2	230 mm of reinforced concrete													
	FILL													
	sand, brown, moist		1AUNDIST											
			1BUNDIST											
			2AUNDIST											
			3AUNDIST											
1.8	SILT													
	trace clay, brown, wet		2BUNDIST											
2.1	SILTY CLAY													
	grey, wet		3AUNDIST											
			3BUNDIST											
			4AUNDIST											
	saturated		4BUNDIST											
			5AUNDIST											
	trace gravel		5BUNDIST											
5.5	END OF BOREHOLE													
	1. Sample refusal at 6.7 m. 2. 50mm-diameter monitoring well installed.		6AUNDIST											

GROUNDWATER ELEVATIONS

Shallow Single Installation  Deep Dual Installation 

GRAPH NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

PROJECT: Mr. Christie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/08/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

SOIL PROFILE		SAMPLES				DYNAMIC CONE PENETRATION		PLASTIC, NAT'L, LIQUID		POCKET PEN.		REMARKS	
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" BLOWS 0.3 m	GROUND WATER CONDITIONS	ELEVATION	SHEAR STRENGTH (kPa) ○ UNCONFINED ● QUICK TRIAXIAL	FIELD VANE + FIELD VANE × LAB VANE	W _p PLASTIC LIMIT	W _L LIQUID LIMIT	NATURAL UNIT WT (G/CM ³)	GRAIN SIZE DISTRIBUTION (%)
25.6	ASPHALT 75 mm of asphalt		1AUNDIST				25	50		10	20		
24.9	FILL sandy silt, brown, moist							100					
0.8	FILL clayey silt, brown, moist		1BUNDIST					150					
24.1	CLAYEY SILT trace sand, brown, moist		2AUNDIST				24	200					
22.1	strong PNC odour between 3-4.5 m trace gravel		2BUNDIST				23	250					
3.5	SANDY SILT brown		3AUNDIST				22						
21.8	SILT/CLAY brown/gray, moist		3BUNDIST				21						
19.6	SHALE slate grey		4AUNDIST				20						
6.7	1 Sample retrieved at 6.7 m 2 Borehole backfilled with bentonite upon completion.		5AUNDIST				19						

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/08/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER		RESISTANCE PLOT					W _p	W _L			
			TYPE	BLOWS N ₆₀	20	40	60	80	100	WATER CONTENT (%)				
					SHEAR STRENGTH (kPa)					FIELD VANE				
					QUICK TRIAXIAL					LAB VANE				
					50	100	150	200	250	10	20	30		
25.7	ASPHALT													
25.6	100 mm of asphalt													
25.7	FILL													
25.7	sand, trace silt, brown, moist		1AUNDIST											
24.0	CLAYEY SILT													
1.7	clayey silt, trace sand, brown, moist		18JUNDIST											
			2AUNDIST											
22.6	SILTY CLAY													
3.0	silty clay, brown, moist		28JUNDIST											
			3AUNDIST											
21.1	CLAYEY SILT													
4.6	clayey silt, brown, moist		38JUNDIST											
			4AUNDIST											
19.0	SHALE													
6.1	shale, grey		48JUNDIST											
			5AUNDIST											
6.7	END OF BOREHOLE													
	1. Sample refusal at 6.7 m. 2. Borehole backfilled with bentonite upon completion.													

GROUNDWATER ELEVATIONS

Shallow Single Installation: Deep Dual Installation:

GRAPH NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/08/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER		RESISTANCE PLOT					W _p	W _L			
			TYPE	BLOWS N ₆₀	20	40	60	80	100	WATER CONTENT (%)				
					SHEAR STRENGTH (kPa)					FIELD VANE				
					QUICK TRIAXIAL					LAB VANE				
					50	100	150	200	250	10	20	30		
25.6	ASPHALT													
25.6	50 mm of asphalt													
25.6	CONCRETE													
25.6	125 mm of concrete													
25.6	FILL													
25.6	silt, trace sand, grey, moist		1AUNDIST											
24.4	Trace clay													
1.2	SILT													
	trace sand, trace clay, brown, moist		18JUNDIST											
			2AUNDIST											
			28JUNDIST											
			3AUNDIST											
			38JUNDIST											
			4AUNDIST											
21.4	SILTY CLAY													
4.3	grey, wet		48JUNDIST											
	some clay, trace sand, wet		5AUNDIST											
			58JUNDIST											
	saturated		6AUNDIST											
18.9	END OF BOREHOLE													
6.7	1. Borehole backfilled with bentonite upon completion.													

GROUNDWATER ELEVATIONS

Shallow Single Installation: Deep Dual Installation:

GRAPH NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

PROJECT: Mr. Chrislie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/08/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (kg / cm²)	NATURAL UNIT WT (Mg/m³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DEPTH	DESCRIPTION	STRATA PLOT		RESISTANCE PLOT					W _p	W _L			
			NUMBER	TYPE	SHEAR STRENGTH (kPa)					WATER CONTENT (%)				
				N°	FIELD VANE									
					QUICK TRIAXIAL									
					20	40	60	80	100	10	20	30		
					50	100	150	200	250					
25.7		ASPHALT												
25.7		100 mm of asphalt												
25.7		FILL	1AUNDIST											
25.7		sand and gravel, brown, moist												
23.9		trace sand, some clay	2AUNDIST											
23.9		CLAY	2BUNDIST											
1.8		dark brown, moist												
2.1		satuated	4AUNDIST											
2.1		SILTY-CLAY	4BUNDIST											
4.6		brown, wet												
		satuated	5AUNDIST											
			5BUNDIST											
19.0		SHALE	6AUNDIST											
18.7		shale, grey												
7.0		END OF BOREHOLE												
		1. Sample refusal at 7.0 m.												
		2. Borehole backfilled with bentonite upon completion.												

GROUNDWATER ELEVATIONS

Shallow Single Installation  Deep Dual Installation 

GRAB NOTES

+ 3, - 3, X, 3: Numbers refer to Sensitivity

○ 5-3% Span at Failure

PROJECT: Mr. Chrislie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/08/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (kg / cm²)	NATURAL UNIT WT (Mg/m³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DEPTH	DESCRIPTION	STRATA PLOT		RESISTANCE PLOT					W _p	W _L			
			NUMBER	TYPE	SHEAR STRENGTH (kPa)					WATER CONTENT (%)				
				N°	FIELD VANE									
					QUICK TRIAXIAL									
					20	40	60	80	100	10	20	30		
					50	100	150	200	250					
25.6		ASPHALT												
25.6		100 mm of asphalt												
25.6		FILL	1AUNDIST											
25.6		sand and gravel, brown, moist												
24.4		SILT	1BUNDIST											
1.2		trace sand, grey/brown, moist												
2.1		CLAY	2AUNDIST											
2.1		dark grey, moist	2BUNDIST											
2.4		SILT	3AUNDIST											
2.4		trace sand, brown, satuated												
		some clay	3BUNDIST											
22.0		CLAYEY SILT	4AUNDIST											
3.7		clayey silt, brown, wet												
20.8		SILTY CLAY	4BUNDIST											
4.9		silty clay, grey, wet	5AUNDIST											
			5BUNDIST											
18.6		END OF BOREHOLE	6AUNDIST											
		1. Borehole backfilled with bentonite upon completion.												

GROUNDWATER ELEVATIONS

Shallow Single Installation  Deep Dual Installation 

GRAB NOTES

+ 3, - 3, X, 3: Numbers refer to Sensitivity

○ 5-3% Span at Failure

SOIL PROFILE		SAMPLES		DYNAMIC CONE PENETRATION PRESSURE PLOT		NATURAL MOISTURE CONTENT		POCKET E (Cu (PSI))		REMARKS AND GRAIN SIZE DISTRIBUTION (%)	
(in) ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" BLOWS 0.3 m	ELEVATION				NATURAL UNIT WT (Mg/m ³)	
25.5	ASPHALT FILL 75 mm of asphalt sand and gravel, trace silt, brown		1AUNDIST			25					
24.9	FILL silt, some sand, grey/brown, moist		1BUNDIST			24					
24.6	trace clay		2AUNDIST			23					
23.1	SILT some clay, brown, moist		2BUNDIST			22					
22.5	SILTY CLAY grey, wet		3AUNDIST			21					
3.0	brown, moist		3BUNDIST			20					
18.6	END OF BOREHOLE 1 Borehole backfilled with bentonite upon completion.		5AUNDIST			19					

SP SOIL LOG 1889-220 BOREHOLE LOGS SET 4.GPJ SP.LGT.GDT 12/5/13												
SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS			DYNAMIC CONE PENETRATION			REMARKS	
(m) ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" BLOWS 0.3	ELEVATION	SHEAR STRENGTH (kPa) ○ LINCOMF-NED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE 50 100 150 200 250	PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT W _L	POCKET PEN. (psi, kPa)	GRAIN SIZE DISTRIBUTION (%)
25.6 28.6 0.2	ASPHALT 1.50 mm of asphalt FILL sand and gravel, brown, moist		1A	UNDIST		25						
			1B	UNDIST		24						
			2A	UNDIST		23						
23.3 2.3	SILTY CLAY brown, moist		2B	UNDIST		23						
22.1 3.5	CLAYEY SILT some sand, grey, moist		3A	UNDIST		22						
20.7 4.9	SILTY CLAY silty clay, grey/brown, moist		4A	UNDIST		21						
19.5 6.1	SHALE slate, grey		4B	UNDIST		20						
18.9 6.7	END OF BOREHOLE 1. Sample refused at 6.7 m 2. Borehole backfilled with bentonite upon completion.		5A	UNDIST		19						

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/11/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS		DYNAMIC CONE PENETRATION		PLASTIC MOISTURE		POCKET PEN.		REMARKS	
(m)	DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS N	ELEVATION	SHEAR STRENGTH (kPa) ○ UNCONF-NED ● QUICK TRIAXIAL	FIELD VANE X LAB VANE	WATER CONTENT (%)	NATURAL MOISTURE LIMIT w _p	LIQUID LIMIT w _L	GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
26.1	0.1	ASPHALT		1AUNDIST			25						
24.4	0.8	FILL sand and gravel, brown, moist		1BUNDIST			24						
23.6	1.5	SILTY CLAY trace sand, brown, moist		2AUNDIST			23						
22.1	3.0	SILT some clay, trace sand, brown, moist		2BUNDIST			22						
20.6	4.6	SILTY CLAY trace sand, brown, wet		3AUNDIST			21						
19.8	5.3	CLAYEY SILT TILL trace gravel, grey, shale fragments		4AUNDIST			20						
19.1	6.1	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.											

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH NOTES: + 3, - 3, Numbers refer to Sensitivity ○ 5-3% Span at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/08/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS		DYNAMIC CONE PENETRATION		PLASTIC MOISTURE		POCKET PEN.		REMARKS	
(m)	DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS N	ELEVATION	SHEAR STRENGTH (kPa) ○ UNCONF-NED ● QUICK TRIAXIAL	FIELD VANE X LAB VANE	WATER CONTENT (%)	NATURAL MOISTURE LIMIT w _p	LIQUID LIMIT w _L	GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
26.1	0.1	ASPHALT		1AUNDIST			24						
24.4	0.8	FILL sand and gravel		1BUNDIST			23						
23.6	1.2	FILL silt, red and grey, wet		2AUNDIST			22						
22.8	1.8	SILT trace clay, brown		2BUNDIST			21						
21.3	3.4	SILTY CLAY grey, wet		3AUNDIST									
20.4	4.3	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.		4AUNDIST									

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH NOTES: + 3, - 3, Numbers refer to Sensitivity ○ 5-3% Span at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/11/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE			SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION										POCKET PEN. (Cu) (PSI)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																			
(m)	ELEV. DEPTH	DESCRIPTION	NUMBER	TYPE			"N" 0.3 m	20	40	60	80	100	120	140	160	180				200	220	240	260	280	300	W _p	NATURAL CONTENT W	LIQUID LIMIT W _L																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
24.5		ASPHALT																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH NOTES: + 3, - 3: Numbers refer to Sensitivity. Strain at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/11/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE			SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT	PLASTIC LIMIT w_p	NATURAL CONTENT w	LIQUID LIMIT w_L	POCKET PEN. (Coi) (kPa)	NATURAL UNIT WT (Mg/m^3)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	ELEV. DEPTH	DESCRIPTION	NUMBER	TYPE									
24.7	0.0	TOPSOIL topsoil, dark brown, moist		1A	UNDIST		24						
24.1	0.6	FILL silt & sand, trace gravel, trace brick, brown, moist		1B	UNDIST								
23.5	1.2	FILL sand & silt, grey/brown, saturated		2A	UNDIST		23						
22.6	2.1	CLAYEY SILT grey/brown, moist		2B	UNDIST								
22.3	2.4	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.											

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH NOTES: + 3, - 3: Numbers refer to Sensitivity. Strain at Failure

PROJECT: Mr. Chrislie	DRILLING DATA	Method: Geo Probe	REF. NO.: 1889-220
CLIENT: [REDACTED]		Diameter:	
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON		Date: Nov/11/2013	
DATUM: Local			ENCL. NO.:
BH LOCATION:			

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS		DYNAMIC CONE PENETRATION		PLASTIC MOISTURE		POCKET PEN.		REMARKS	
(m)	ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS N ₆₀							
24.7		ASPHALT											
24.1		FILL											
0.3		75 mm of asphalt											
		SILT											
		some clay, trace sand, gravel, brown, moist											
				1AUNDIST									
				1BUNDIST									
				1CUNDIST									
				2AUNDIST									
				2BUNDIST									
				2CUNDIST									
				3AUNDIST									
21.7		CLAYEY SILT											
3.0		grey, moist											
				3AUNDIST									
				3BUNDIST									
				3CUNDIST									
				4AUNDIST									
				4BUNDIST									
				4CUNDIST									
19.8		trace shale fragments											
4.8		END OF BOREHOLE											
		1. Borehole backfilled with bentonite upon completion.											

GROUNDWATER ELEVATIONS

Shallow Single Installation: Deep Dual Installation:

GRAPH NOTES

+ 3' x 3' : Numbers refer to Sensitivity

○ 5-3% Strain at Failure

PROJECT: Mr. Chrislie	DRILLING DATA	Method: Geo Probe	REF. NO.: 1889-220
CLIENT: [REDACTED]		Diameter:	
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON		Date: Nov/08/2013	
DATUM: Local			ENCL. NO.:
BH LOCATION:			

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS		DYNAMIC CONE PENETRATION		PLASTIC MOISTURE		POCKET PEN.		REMARKS	
(m)	ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS N ₆₀							
25.6		ASPHALT											
24.1		FILL											
20.1		75 mm of asphalt											
		SILT											
		sand and gravel, brown, moist											
				1AUNDIST									
				1BUNDIST									
				1CUNDIST									
				2AUNDIST									
				2BUNDIST									
				2CUNDIST									
				3AUNDIST									
				3BUNDIST									
				3CUNDIST									
22.4		dark grey											
3.2		SILT											
		trace clay, gravel, grey/brown, moist											
				3AUNDIST									
				3BUNDIST									
				3CUNDIST									
21.0		SILTY CLAY											
4.6		grey, moist											
				4AUNDIST									
				4BUNDIST									
				4CUNDIST									
19.7		CLAY											
18.9		grey, moist											
6.1		SHALE											
		grey											
18.9													
6.7		END OF BOREHOLE											
		1. Sample refusal at 6.7 m.											
		2. 50mm-diameter monitoring well installed.											

GROUNDWATER ELEVATIONS

Shallow Single Installation: Deep Dual Installation:

GRAPH NOTES

+ 3' x 3' : Numbers refer to Sensitivity

○ 5-3% Strain at Failure

PROJECT: Mr. Christie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/08/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

(m) ELEV DEPTH	SOIL PROFILE DESCRIPTION	SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (T)	PLASTIC LIMIT	NATURAL CONTENT	POCKET PEN. (kg / cm²)	NATURAL UNIT WT (Mg/m³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
		STRATA PLOT NUMBER	TYPE				W _p	W			
25.6	ASPHALT										
25.3	100 mm of asphalt										
25.2	FILL sand and gravel, brown, moist		1AUNDIST		25						
24.3	FILL sand & silt, grey/brown, moist		1BUNDIST								
1.2	FILL silty clay, grey, wet		2AUNDIST		24						
1.8	CLAYEY SILT trace organics, brown, moist		2BUNDIST								
22.5	grey/brown		3AUNDIST		23						
3.0	SILTY CLAY grey, moist		3BUNDIST		22						
20.7	wet		4AUNDIST		21						
4.9	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.		4BUNDIST		20						

GROUNDWATER ELEVATIONS

Shallow Single Installation Deep Dual Installation

GRAPH
NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 3.GPJ SPL.GDT 12/5/13

PROJECT: Mr. Christie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/08/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

(m) ELEV DEPTH	SOIL PROFILE DESCRIPTION	SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (T)	PLASTIC LIMIT	NATURAL CONTENT	POCKET PEN. (kg / cm²)	NATURAL UNIT WT (Mg/m³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
		STRATA PLOT NUMBER	TYPE				W _p	W			
24.7	TOPSOIL										
24.6	1.0 m Topsoil		1AUNDIST		24						
0.3	FILL sand, brown, moist		1BUNDIST								
23.2	SILTY CLAY trace sand, organics, grey, wet		2AUNDIST		23						
2.4	CLAYEY SILT clayey silt, brown, moist		3AUNDIST		22						
20.5	trace sand, trace shale, some gravel		3BUNDIST		21						
4.3	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.		4AUNDIST								

GROUNDWATER ELEVATIONS

Shallow Single Installation Deep Dual Installation

GRAPH
NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/11/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION RESISTANCE (TOL)					PLASTIC MOISTURE LIMIT		NATURAL MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT NUMBER	TYPE		20	40	60	80	100	w _p	w _L	w	w _L			
24.6	ASPHALT															
24.3	100 mm of asphalt		1AUNDIST													
24.0	FILL sand and gravel															
0.3	FILL silt, some clay, grey, moist		1BUNDIST													
	grey/brown															
			2AUNDIST													
22.5	CLAY clay, brick and glass fragments															
2.1	wet		2BUNDIST													
22.2	END OF BOREHOLE															
2.4	1. Sample refusal at 2.4 m. 2. Borehole backfilled with bentonite upon completion.															

GROUNDWATER ELEVATIONS
Shallow Single Installation Deep/Dual Installation

GRAPH NOTES + 3, x 3: Numbers refer to Sensitivity 5-3% Strain at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/11/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION RESISTANCE (TOL)					PLASTIC MOISTURE LIMIT		NATURAL MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT NUMBER	TYPE		20	40	60	80	100	w _p	w _L	w	w _L			
24.8	TOPSOIL															
0.0	topsoil, dark brown, moist		1AUNDIST													
24.5	FILL															
0.3	silt, brown, moist		1BUNDIST													
24.2	SILT															
0.6	some clay, trace sand, brown, moist		1BUNDIST													
			2AUNDIST													
			2AUNDIST													
			2BUNDIST													
	oxidation															
			3AUNDIST													
	grey, wet															
			3BUNDIST													
			4AUNDIST													
			4BUNDIST													
20.0	END OF BOREHOLE															
4.8	1. Borehole backfilled with bentonite upon completion.															

GROUNDWATER ELEVATIONS
Shallow Single Installation Deep/Dual Installation

GRAPH NOTES + 3, x 3: Numbers refer to Sensitivity 5-3% Strain at Failure

PROJECT: Mr. Chrislie	DRILLING DATA	Method: Geo Probe	REF. NO.: 1889-220
CLIENT: [REDACTED]		Diameter:	
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON		Date: Nov/12/2013	
DATUM: Local			ENCL. NO.:
BH LOCATION:			

(m) ELEV DEPTH	SOIL PROFILE DESCRIPTION	SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (T)					PLASTIC MOISTURE LIMIT		NATURAL MOISTURE CONTENT		POCKET PEN. (kg / cm²)	NATURAL UNIT WT (Mg/m³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
		STRATA PLOT	NUMBER TYPE			20	40	60	80	100	w _p	w _L	w	w _L			
25.5	TOPSOIL		1AUNDIST														
28.9	SILT																
0.2	some clay, dark brown, moist																
24.0	CLAYEY SILT		1BUNDIST														
1.5	brown, wet																
			2AUNDIST														
			2BUNDIST														
			3AUNDIST														
21.2	SILTY CLAY		3BUNDIST														
3.4	grey, saturated																
20.9																	
4.6	END OF BOREHOLE																
	1. Borehole backfilled with bentonite upon completion.																

GROUNDWATER ELEVATIONS

Shallow Single Installation Deep Dual Installation

GRAPH NOTES

+ 3, - 3: Numbers refer to Sensitivity Span at Failure

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 3.GPJ SPL.GDT 12/5/13

PROJECT: Mr. Chrislie	DRILLING DATA	Method: Geo Probe	REF. NO.: 1889-220
CLIENT: [REDACTED]		Diameter:	
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON		Date: Nov/08/2013	
DATUM: Local			ENCL. NO.:
BH LOCATION:			

(m) ELEV DEPTH	SOIL PROFILE DESCRIPTION	SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (T)					PLASTIC MOISTURE LIMIT		NATURAL MOISTURE CONTENT		POCKET PEN. (kg / cm²)	NATURAL UNIT WT (Mg/m³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
		STRATA PLOT	NUMBER TYPE			20	40	60	80	100	w _p	w _L	w	w _L			
25.6	ASPHALT		1AUNDIST														
23.2	FILL																
0.3	150 mm of asphalt																
25.0	sand and gravel, brown, moist																
0.6	FILL																
	sand, brown, moist																
	SILT		1BUNDIST														
	trace sand, some clay, brown, moist																
24.1	CLAYEY SILT		2AUNDIST														
1.5	grey, moist																
			2BUNDIST														
			3AUNDIST														
			3BUNDIST														
	grey/brown, wet																
			4AUNDIST														
20.7	moist		4BUNDIST														
4.9	END OF BOREHOLE																
	1. Borehole backfilled with bentonite upon completion.																

GROUNDWATER ELEVATIONS

Shallow Single Installation Deep Dual Installation

GRAPH NOTES

+ 3, - 3: Numbers refer to Sensitivity Span at Failure

PROJECT: Mr. Christie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATE/TIME: Local	Date: Nov/12/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

SOIL PROFILE			SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT	PLASTIC LIMIT NATURAL WATER CONTENT (%)	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)		
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE									
25.6	TOPSOIL		1AUNDIST		25			10 20 30			GR SA SI CL		
25.4	wet		1BUNDIST										
23.4	SILTY CLAY		2AUNDIST									24	
23	grey, moist		2BUNDIST										22
			3AUNDIST										
			3BUNDIST										
21.1	END OF BOREHOLE												
4.6	1. Borehole backfilled with Bentonite upon completion.												

[illegible]

GROUNDWATER ELEVATIONS	GRAPH NOTES	○ 2-3% Strain at Failure
Shallow Single Installation: ∇ 	+ , × : Numbers refer to Sensitivity	
Deep Dual Installation: ∇ 		

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 6.GPJ SPL.GDT 12/5/13

PROJECT: M. Christie
CLIENT:
METHOD: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd., Toronto, ON
DATE/TIME: Local
BH LOCATION:

REF. NO.: 1889-220
ENCL. NO.:

DRILLING DATA

Method: Geo Probe
Diameter:
Date: Nov/13/2013

(m) ELEV. DEPTH	SOIL PROFILE DESCRIPTION	SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div><div>20406080100</div><div><div><div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><di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GROUNDWATER ELEVATIONS

Shallow Single Installation ∇  

Deep Dual Installation ∇  

GRAPH NOTES

+ , \times 3 Numbers refer to Sensitivity

\circ $\varepsilon=3\%$ Strain at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/13/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		NATURAL MOISTURE		POCKET PEN. (CU. IN. PSI)	NATURAL UNIT WT (MG/CM ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS N	20	40	60	80	100	W _p	W	W _L			
25.7	FILL sand and gravel, brown, moist															
24.1	FILL silt, brown, moist, loose															
24.1	occasional brick															
24.1	grey, very moist															
1.5	END OF BOREHOLE															
1.5	1. Borehole backfilled with bentonite upon completion															

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:

GRAPH NOTES
+ 3, - 3: Numbers refer to Sensitivity
○ 5-3%: Strain at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/13/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		NATURAL MOISTURE		POCKET PEN. (CU. IN. PSI)	NATURAL UNIT WT (MG/CM ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS N	20	40	60	80	100	W _p	W	W _L			
25.7	ASPHALT															
24.2	FILL sand and gravel															
24.2	SILT trace sand, oxidation, brown, moist															
24.2																
1.5	END OF BOREHOLE															
1.5	1. Sample refusal at 1.5 m 2. Borehole backfilled with bentonite upon completion.															

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:

GRAPH NOTES
+ 3, - 3: Numbers refer to Sensitivity
○ 5-3%: Strain at Failure

[illegible]

[illegible]

PROJECT: Mr. Christie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/13/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

(m) ELEV. DEPTH	SOIL PROFILE DESCRIPTION	SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (PLT)					PLASTIC MOISTURE LIMIT		NATURAL MOISTURE CONTENT		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
		STRATA PLOT NUMBER	TYPE			20	40	60	80	100	w _p	w _L	w	w _L			
25.8	TOPSOIL																
24.7	SILT dark brown, moist some clay, trace sand, trace gravel, brown, moist very moist	1A			25												
		1B															
		2A			24												
	organic layer at 1.4 m (thickness 200 mm)	2A															
		2B															
		3A			23												
		3B															
21.9	CLAYEY SILT	4A			22												
4.0	clayey silt, grey, very moist wet	4A															
		4B															
20.9																	
4.9	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.				21												

GROUNDWATER ELEVATIONS

Shallow Single Installation Deep Dual Installation

GRAPH
NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 6.GPJ SPL.GDT 12/5/13

PROJECT: Mr. Christie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/12/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

(m) ELEV. DEPTH	SOIL PROFILE DESCRIPTION	SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (PLT)					PLASTIC MOISTURE LIMIT		NATURAL MOISTURE CONTENT		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
		STRATA PLOT NUMBER	TYPE			20	40	60	80	100	w _p	w _L	w	w _L			
25.8	TOPSOIL																
24.9	SILT 150 mm, dark brown, wet, loose silt, some clay, trace sand and gravel, greyish brown, wet, loose		UNDIST		25												
0.2	FILL																
24.3	SILT some clay, brown, moist, hard occasional grey fissures		UNDIST		24												
			UNDIST														
			UNDIST		23												
			UNDIST														
21.9	SILTY CLAY grey, wet, soft		UNDIST		22												
4.0			UNDIST														
			UNDIST		21												
20.3	shale																
5.5	END OF BOREHOLE 1. 50 mm-diameter monitoring well installed 2. Water encountered at 0.9 mbg Nov. 12, 2013.																

GROUNDWATER ELEVATIONS

Shallow Single Installation Deep Dual Installation

GRAPH
NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

PROJECT: Mr. Christie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/12/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

(m) ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRATA PLOT	SAMPLES NUMBER	TYPE	BLOWS N 0.3	GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w		LIQUID LIMIT w _L	POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
								20	40	60	80	100		10	20	30			
25.8	ASPHALT																		
25.2	FILL		1A	UNDIST															
0.6	25.2 silt, some clay, trace sand and gravel, brown, moist SILTY CLAY																		
0.6	zones of oxide ion, brown, moist																		
			1B	UNDIST															
			2A	UNDIST															
			2B	UNDIST															
22.8	SAND																		
3.0	trace silt, brownish grey, saturated clayey silt, grey																		
			3A	UNDIST															
			3B	UNDIST															
21.3	SILTY CLAY																		
4.6	grey, wet, soft																		
			4A	UNDIST															
			4B	UNDIST															
19.7	shale fragments																		
			4B	UNDIST															
6.1	END OF BOREHOLE																		
	1. Borehole backfilled with bentonite upon completion.																		

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 6.GPJ SPL.GDT 12/5/13

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH NOTES: + 3, - 3: Numbers refer to Sensitivity. Strain at Failure

PROJECT: Mr. Christie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/13/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

(m) ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRATA PLOT	SAMPLES NUMBER	TYPE	BLOWS N 0.3	GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w		LIQUID LIMIT w _L	POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
								20	40	60	80	100		10	20	30			
25.8	ASPHALT																		
25.2	FILL		1A	UNDIST															
0.1	1.25 mm of asphalt																		
0.8	FILL																		
25.1	sand and gravel, brown, moist																		
			1B	UNDIST															
			2A	UNDIST															
24.3	CLAYEY SILT																		
1.5	clayey silt, trace sand, brown, damp																		
			2A	UNDIST															
			2B	UNDIST															
			3A	UNDIST															
			3B	UNDIST															
21.3	damp/moist																		
			3B	UNDIST															
4.6	END OF BOREHOLE																		
	1. Sample refusal at 4.6 m. 2. Borehole backfilled with bentonite upon completion.																		

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 7.GPJ SPL.GDT 12/5/13

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH NOTES: + 3, - 3: Numbers refer to Sensitivity. Strain at Failure

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION		PLASTIC LIMIT		POCKET PEN. (Cu (kPa))	REMARKS
(m) ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER			TYPE	BLOWS 0.3 m	20 40 60 80 100	10 20 30		
25.7	ASPHALT		1AUNDIST		25						
25.0	FILL sand and gravel										
0.6	FILL silt, some clay, trace sand and gravel, brown, moist, compact		1BUNDIST								
24.4	SILT some clay, grey, moist, hard		2AUNDIST		24						
20.8	clayey silt, occasional silt layers		2BUNDIST		23						
			3AUNDIST		22						
			3BUNDIST		21						
			4AUNDIST								
			4BUNDIST								
4.9	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion										

GRAPH + 3, $\times 3$: Numbers refer to Sensitivity ☐ $\epsilon = 3\%$ Strain at Failure

NOTES

1 OF 1

[illegible]

GRAPH + 3 × 3: Numbers refer to Sensitivity ○ $\epsilon = 3\%$ Strain at Failure

NOTES

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/13/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION RESISTANCE (T)					PLASTIC MOISTURE LIMIT		NATURAL CONTENT		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DEPTH	DESCRIPTION	STRATA PLOT NUMBER	TYPE	BLOWS N	20	40	60	80	100	w _p	w	w _L			
25.8		ASPHALT														
25.7		FILL	1AUNDIST													
25.0		fill sand and gravel, brown, damp														
0.8		FILL clayey silt, trace sand, trace gravel, brown, moist	1BUNDIST													
24.3		SILTY CLAY														
1.5		silty clay, trace sand, grey, very moist	2AUNDIST													
23.7		CLAYEY SILT														
2.1		clayey silt, trace sand, brown, moist	2BUNDIST													
		brown/grey	3AUNDIST													
			3BUNDIST													
21.2		END OF BOREHOLE														
4.6		1. Borehole backfilled with bentonite upon completion.														

GROUNDWATER ELEVATIONS

Shallow Single Installation Deep Dual Installation

GRAIN
NOTES

+ 3 : 3 Numbers refer to Sensitivity
○ 5-3% Span at Failure

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 6.GPJ SPL.GDT 12/5/13

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/13/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION RESISTANCE (T)					PLASTIC MOISTURE LIMIT		NATURAL CONTENT		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DEPTH	DESCRIPTION	STRATA PLOT NUMBER	TYPE	BLOWS N	20	40	60	80	100	w _p	w	w _L			
25.7		ASPHALT														
25.1		FILL	1AUNDIST													
0.6		fill sand and gravel, brown, moist														
24.2		FILL silty clay, trace organics, grey, moist, soft	1BUNDIST													
1.5		END OF BOREHOLE														
		1. Borehole backfilled with bentonite upon completion.														

GROUNDWATER ELEVATIONS

Shallow Single Installation Deep Dual Installation

GRAIN
NOTES

+ 3 : 3 Numbers refer to Sensitivity
○ 5-3% Span at Failure

1001

[illegible]

1 OF 1

1001

[illegible]

PROJECT: Mr. Christie	DRILLING DATA	Method: Geo Probe	REF. NO.: 1889-220
CLIENT: [REDACTED]		Diameter:	
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON		Date: Nov/13/2013	
DATUM: Local			ENCL. NO.:
BH LOCATION:			

(m) ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRATA PLOT		SAMPLES NUMBER TYPE BLOWS N	GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W		POCKET PEN. C _u (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
							20	40	60	80	100		10	20	30		
25.8	ASPHALT 75 mm of asphalt			1AUNDIST		25											
24.1	FILL sand and gravel, brown, damp brick fragments, asphalt fragments, grey			1BUNDIST													
24.2	FILL clayey silt, some sand, trace organics, black/brown, moist			2AUNDIST		24											
1.5																	
22.7	CLAYEY SILT brown, moist/very moist			2BUNDIST		23											
3.0				3AUNDIST		22											
2.2				3BUNDIST													
4.6	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.																

GROUNDWATER ELEVATIONS

Shallow Single Installation: Deep Dual Installation:

GRAPH
NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3%: Strain at Failure

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 6.GPJ SPL.GDT 12/5/13

PROJECT: Mr. Christie	DRILLING DATA	Method: Geo Probe	REF. NO.: 1889-220
CLIENT: [REDACTED]		Diameter:	
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON		Date: Nov/13/2013	
DATUM: Local			ENCL. NO.:
BH LOCATION:			

(m) ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRATA PLOT		SAMPLES NUMBER TYPE BLOWS N	GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W		POCKET PEN. C _u (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
							20	40	60	80	100		10	20	30		
25.8	ASPHALT 100 mm of asphalt			1AUNDIST		25											
25.2	FILL sand, trace gravel, brown, moist			1BUNDIST													
0.6	FILL silt, trace clay and sand, brown, moist			2AUNDIST		24											
	grey			2BUNDIST													
23.3				3AUNDIST		23											
2.4	CLAYEY SILT occasional sand layer, grey, moist			3BUNDIST		22											
20.9				4AUNDIST													
4.9	SHALE			4BUNDIST		21											
20.3	END OF BOREHOLE 1. Sample refusal at 5.5 m. 2. Borehole backfilled with bentonite upon completion.			5AUNDIST													

GROUNDWATER ELEVATIONS

Shallow Single Installation: Deep Dual Installation:


GRAPH
NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3%: Strain at Failure

[illegible]

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 7.GPJ SPL.GDT 12/5/13

GROUNDWATER ELEVATIONS

Shallow Single Installation  

Deep Dual Installation  

GRAPH NOTES

$+3 \times 3$: Numbers refer to Sensitivity

$\phi = 3\%$ Strain at Failure

Shallow/ Single Installation Deep/Dual Installation

GRAPH + 3 × 3: Numbers refer to Sensitivity

NOTES

☐ $\epsilon = 3\%$ Strain at Failure

[illegible]

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 6 GPL SPL GDT 12/5/13

GROUNDWATER ELEVATIONS	GRAPH NOTES	
Shallow Single Installation 	+3, $\times 3$; Numbers refer to Sensitivity	<input type="radio"/> $\epsilon = 3\%$ Strain at Failure
Deep Dual Installation 		

Shallow/ Single Installation	Deep/Dual Installation

GRAPH + 3 × 3: Numbers refer to Sensitivity

☐ $\epsilon = 3\%$ Strain at Failure

PROJECT: Mr. Christie	DRILLING DATA	Method: Geo Probe	REF. NO.: 1889-220
CLIENT: [REDACTED]		Diameter:	
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON		Date: Nov/14/2013	ENCL. NO.:
DATUM: Local			
BH LOCATION:			

SOIL PROFILE	SAMPLES			GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (PLT)	PLASTIC MOISTURE LIMIT	NATURAL MOISTURE CONTENT	POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
	(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT NUMBER								
25.5 25.6 25.8 26.0 26.1	ASPHALT 125 mm of asphalt sand and gravel, brown, damp	1AUNDIST			25						
24.8 0.8	FILL clayey silt, trace sand, trace organics, dark brown	1BUNDIST			24						
23.1	CLAYEY SILT clayey silt, trace sand, brown, moist	2AUNDIST			23						
21.0 20.8 20.6 20.4 20.2	SHALE shale fragments, grey	2BUNDIST 3AUNDIST 3BUNDIST 4AUNDIST			22						
1.5	END OF BOREHOLE 1. Sample refusal at 4.7 m. 2. 50mm-diameter monitoring well installed at 5.3 m.										

GROUNDWATER ELEVATIONS
Shallow Single Installation: [Symbol] Deep/Dual Installation: [Symbol]

GRAIN SIZE NOTES
+ 3, - 3: Numbers refer to Sensitivity
○ 5-3%: Spinn at Failure

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 6.GPJ SPL.GDT 12/5/13

PROJECT: Mr. Christie	DRILLING DATA	Method: Geo Probe	REF. NO.: 1889-220
CLIENT: [REDACTED]		Diameter:	
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON		Date: Nov/13/2013	ENCL. NO.:
DATUM: Local			
BH LOCATION:			

SOIL PROFILE	SAMPLES			GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE (PLT)	PLASTIC MOISTURE LIMIT	NATURAL MOISTURE CONTENT	POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
	(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT NUMBER								
25.5 25.6 25.8 26.0 26.1	ASPHALT 90 mm of asphalt sand and gravel, brown, moist	1AUNDIST			25						
24.9 0.8	FILL silt mixed with slag, brown, moist	1BUNDIST									
1.5	END OF BOREHOLE 1. Sample refusal at 1.5 m. 2. Borehole backfilled with bentonite upon completion.										

GROUNDWATER ELEVATIONS
Shallow Single Installation: [Symbol] Deep/Dual Installation: [Symbol]

GRAIN SIZE NOTES
+ 3, - 3: Numbers refer to Sensitivity
○ 5-3%: Spinn at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE			SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	ELEV. / DEPTH	DESCRIPTION	STRATA PLOT	NUMBER		RESISTANCE PLOT					W _p	W _L			
						20	40	60	80	100	NATURAL MOISTURE				
						SHEAR STRENGTH (kPa)					WATER CONTENT (%)				
						● UNCONF-NED					FIELD VANE				
						○ QUICK TRIAXIAL					LAB VANE				
						50	100	150	200	250	10	20	30		
25.6		ASPHALT													
24.7		FILL		1AUNDIST											
		sand and gravel, brown, damp													
24.8		FILL													
0.8		clayey silt, some sand, brown, moist		1BUNDIST											
24.2		CLAYEY SILT													
1.4		trace sand, brown, damp/moist		2AUNDIST											
				2AUNDIST											
22.5		CLAYEY SILT TILL													
3.0		silt and shale fragments, trace gravel, brown, moist		2BUNDIST											
				3AUNDIST											
		grey													
				3BUNDIST											
2.0															
4.6		END OF BOREHOLE													
		1. Borehole backfilled with bentonite upon completion.													

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 7.GPJ SPL.GDT 12/5/13

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH NOTES: + 3, - 3, X 3: Numbers refer to Sensitivity. ○ 5-3%: Strain at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/13/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE			SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	ELEV. / DEPTH	DESCRIPTION	STRATA PLOT	NUMBER		RESISTANCE PLOT					W _p	W _L			
						20	40	60	80	100	NATURAL MOISTURE				
						SHEAR STRENGTH (kPa)					WATER CONTENT (%)				
						● UNCONF-NED					FIELD VANE				
						○ QUICK TRIAXIAL					LAB VANE				
						50	100	150	200	250	10	20	30		
25.6		FILL		1AUNDIST											
0.0		sand and gravel, brown with black, moist													
24.8		FILL													
0.8		silt, some clay, greyish brown, moist		1BUNDIST											
24.1															
1.5		END OF BOREHOLE													
		1. Borehole backfilled with bentonite upon completion.													

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 6.GPJ SPL.GDT 12/5/13

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH NOTES: + 3, - 3, X 3: Numbers refer to Sensitivity. ○ 5-3%: Strain at Failure

PROJECT: Mr. Christie
CLIENT: ██████████
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION: _____

DRILLING DATA
Method: Geo Probe
Diameter: _____
Date: Nov/14/2013
REF. NO.: 1889-220
ENCL. NO.: _____

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION RESISTANCE (PLT)					PLASTIC MOISTURE LIMIT		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DESCRIPTION	STRATA PLOT NUMBER	TYPE		20	40	60	80	100	W _p	W _L			
25.6	ASPHALT 100 mm of asphalt	1AUNDIST	BLOWS N = 0.3		50	100	150	200	250	10	20	30		GR SA, SI, CL
24.8	FILL sand and gravel, brown, damp	1AUNDIST												
0.8	FILL clayey silt, some sand, some gravel, brown, moist	1BUNDIST												
24.0	CLAYEY SILT trace gravel, brown/grey, moist	2AUNDIST												
		2BUNDIST												
		3AUNDIST												
21.8	grey													
3.8	SILTY CLAY silty clay, trace gravel, grey, very moist	3BUNDIST												
2.0														
4.8	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.													

GROUNDWATER ELEVATIONS

Shallow Single Installation: Deep Dual Installation:

GRAPH
NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 6.GPJ SPL.GDT 12/5/13

PROJECT: Mr. Christie
CLIENT: ██████████
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION: _____

DRILLING DATA
Method: Geo Probe
Diameter: _____
Date: Nov/14/2013
REF. NO.: 1889-220
ENCL. NO.: _____

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION RESISTANCE (PLT)					PLASTIC MOISTURE LIMIT		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DESCRIPTION	STRATA PLOT NUMBER	TYPE		20	40	60	80	100	W _p	W _L			
25.7	FILL gravel, moist	1AUNDIST	BLOWS N = 0.3		50	100	150	200	250	10	20	30		GR SA, SI, CL
0.0	FILL gravel with some sand and silt, brown, moist	1AUNDIST												
25.1	FILL silt with some clay, greyish brown, moist	1BUNDIST												
24.5	FILL silty clay, trace sand, grey, moist	2AUNDIST												
1.2		2AUNDIST												
		2BUNDIST												
23.3	wet													
2.4	END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.													

GROUNDWATER ELEVATIONS

Shallow Single Installation: Deep Dual Installation:

GRAPH
NOTES

+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		NATURAL MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS N ₆₀	ELEVATION	SHEAR STRENGTH (kPa) ○ UNCONF-NED ● QUICK TRIAXIAL	FIELD VANE + LAB VANE	W _p	W _L	W	W _L			
26.0		FILL sand with gravel, trace silt, brown, moist		1AUNDIST												
25.0		FILL silt with some clay, trace sand, brown and grey, wet		1BUNDIST												
24.5		FILL silty clay, grey, wet		2AUNDIST												
23.6		END OF BOREHOLE 1. Borehole backfilled with bentonite upon completion.		2BUNDIST												

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH: + 3, -3: Numbers refer to Sensitivity
NOTES: 5-3% Span at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		NATURAL MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS N ₆₀	ELEVATION	SHEAR STRENGTH (kPa) ○ UNCONF-NED ● QUICK TRIAXIAL	FIELD VANE + LAB VANE	W _p	W _L	W	W _L			
26.3		TOPSOIL		1AUNDIST												
25.5		FILL silt, some clay, some sand, some gravel, brown, moist		1AUNDIST												
25.0		FILL clayey silt, trace sand, trace gravel, brown, moist		1BUNDIST												
24.6		CLAYEY SILT brown/grey, moist		2AUNDIST												
23.6		grey, moist		2BUNDIST												
23.3		grey, moist		3AUNDIST												
22.6		grey, moist		3BUNDIST												
22.0		grey, moist		3BUNDIST												
21.3		grey, moist		3BUNDIST												
20.6		grey, moist		3BUNDIST												
19.3		END OF BOREHOLE 1. Sample refusal at 7.0 m. 2. Sonnet diameter monitoring well installed at 7.6 m.		3AUNDIST												

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:
GRAPH: + 3, -3: Numbers refer to Sensitivity
NOTES: 5-3% Span at Failure

PROJECT: Mr. Chrishe
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		NATURAL MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	ELEV. DEPTH	DESCRIPTION	STRATA PLOT NUMBER	TYPE	BLOWS N	20	40	60	80	100	W _p	W	W _L			
25.7		ASPHALT														
24.8		75 mm of asphalt		1AUNDIST												
24.2		FILL sand, trace silt, brown, saturated		1AUNDIST												
1.5		FILL clayey silt, grey, wet		1BUNDIST												
2.4		FILL		2AUNDIST												
2.4		1. Sample refusal at 2.4 m. 2. Borehole backfilled with bentonite upon completion.		2BUNDIST												
2.4		END OF BOREHOLE														

GROUNDWATER ELEVATIONS
Shallow Single Installation Deep Dual Installation
GRAIN SIZE NOTES
+ 3, x 3, : Numbers refer to Sensitivity
○ 5-3% Span at Failure

PROJECT: Mr. Chrishe
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION					PLASTIC MOISTURE		NATURAL MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	ELEV. DEPTH	DESCRIPTION	STRATA PLOT NUMBER	TYPE	BLOWS N	20	40	60	80	100	W _p	W	W _L			
26.4		TOPSOIL														
2.0		230 mm		1AUNDIST												
2.4		FILL silt with some clay, trace sand, brown, moist.		1AUNDIST												
2.4		FILL		1BUNDIST												
2.4		2.0 grey, wet		2AUNDIST												
2.4		SILT CLAY		2BUNDIST												
2.4		1. Sample refusal at 2.4 m. 2. Borehole backfilled with bentonite upon completion.														
2.4		END OF BOREHOLE														

GROUNDWATER ELEVATIONS
Shallow Single Installation Deep Dual Installation
GRAIN SIZE NOTES
+ 3, x 3, : Numbers refer to Sensitivity
○ 5-3% Span at Failure

PROJECT: Mr. Chishe
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION		PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER		RESISTANCE (T)	SHEAR STRENGTH (kPa)	W _p	NATURAL MOISTURE			
25.7	ASPHALT										
25.8	FILL		1AUNDIST								
0.1	120 mm sand, cinders brown, moist										
			1BUNDIST								
			2AUNDIST								
			2BUNDIST								
23.2	END OF BOREHOLE										
2.4	1. Sample refusal at 2.4 m. 2. Borehole backfilled with bentonite upon completion.										

GROUNDWATER ELEVATIONS
Shallow Single Installation Deep Dual Installation
GRAPH NOTES +3, -3: Numbers refer to Sensitivity Span at Failure

PROJECT: Mr. Chishe
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:
DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013
REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	DYNAMIC CONE PENETRATION		PLASTIC MOISTURE		POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV. DEPTH	DESCRIPTION	STRATA PLOT	NUMBER		RESISTANCE (T)	SHEAR STRENGTH (kPa)	W _p	NATURAL MOISTURE			
26.5	GRAVEL										
26.6	FILL		1AUNDIST								
0.1	130 mm silt, trace sand and clay, brown, very moist										
25.9	SILT		1BUNDIST								
0.6	some clay, trace sand, brown, moist, hard, iron staining		2AUNDIST								
			2BUNDIST								
24.1	silty clay, grey, wet										
2.4	END OF BOREHOLE										
2.4	1. Sample refusal at 4.9 m. 2. Borehole backfilled with bentonite upon completion.										

GROUNDWATER ELEVATIONS
Shallow Single Installation Deep Dual Installation
GRAPH NOTES +3, -3: Numbers refer to Sensitivity Span at Failure

PROJECT: Mr Christie
CLIENT: [REDACTED]
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013

REF. NO.: 1889-220
ENCL NO.:

BH LOCATION:

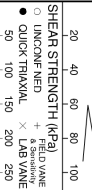
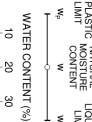
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PROJECT: Mr. Christie
CLIENT: [REDACTED]
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION: [REDACTED]

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013

REF. NO.: 1889-220
ENCL NO.:

BH LOCATION

SOIL PROFILE			SAMPLES				GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION PRESSURE (TS) 	NATURAL PLASTIC MOISTURE CONTENT 	POCKET PEN. (C_u (kPa))	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m) ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" BLOWS 0.3 m							
26.6	TOPSOIL						25					
25.3	trace sand, trace gravel, dark brown, moist		1AUNDIST									
25.3	trace clay, trace sand, trace gravel, brown, moist											
0.3	SILT											
	trace clay, trace sand, trace gravel, some clay, grey, wet											
23.8	SILTY CLAY		2AUNDIST				24					
1.8			2BUNDIST									
23.2	END OF BOREHOLE											
2.4	1. Sample refusal at 2.4 m. 2. Borehole backfilled with bentonite upon completion.											

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 7.GPJ SPL.GDT 12/5/13

GROUNDWATER ELEVATIONS

Shallow/Single Installation   Deep/Dual Installation  

NOTES

+³, ×³: Numbers refer to Sensitivity

☐ $\epsilon = 3\%$ Strain at Failure

GROUNDWATER ELEVATIONS

Shallow/ Single Installation   Deep/Dual Installation  

NOTES

+3, ×3: Numbers refer to Sensitivity

☐ $\epsilon = 3\%$ Strain at Failure

PROJECT: Mr. Christie	DRILLING DATA	REF. NO.: 1889-220
CLIENT: [REDACTED]	Method: Geo Probe	ENC. NO.:
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:	
DATE/LOC: Local	Date: Nov/14/2013	
BH LOCATION:		

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT	PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w _L	POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
DESCRIPTION	STRATA PLOT	NUMBER	TYPE									
25.5 25.2 25.0 24.8 24.6 24.4 24.2 24.0 23.8 23.6 23.4 23.2 23.0 22.8 22.6 22.4 22.2 22.0 21.8 21.6 21.4 21.2 21.0 20.8 20.6 20.4 20.2 20.0 19.8 19.6 19.4 19.2 19.0 18.8 18.6 18.4 18.2 18.0 17.8 17.6 17.4 17.2 17.0 16.8 16.6 16.4 16.2 16.0 15.8 15.6 15.4 15.2 15.0 14.8 14.6 14.4 14.2 14.0 13.8 13.6 13.4 13.2 13.0 12.8 12.6 12.4 12.2 12.0 11.8 11.6 11.4 11.2 11.0 10.8 10.6 10.4 10.2 10.0 9.8 9.6 9.4 9.2 9.0 8.8 8.6 8.4 8.2 8.0 7.8 7.6 7.4 7.2 7.0 6.8 6.6 6.4 6.2 6.0 5.8 5.6 5.4 5.2 5.0 4.8 4.6 4.4 4.2 4.0 3.8 3.6 3.4 3.2 3.0 2.8 2.6 2.4 2.2 2.0 1.8 1.6 1.4 1.2 1.0 0.8 0.6 0.4 0.2 0.0	ASPHALT 75 mm of asphalt	1AUNDIST			25	<div><div>20406080100</div><div>SHEAR STRENGTH (kPa) FIELD VALUE x 1.5 POCKET PEN x 1.5 CLICK TRIAXIAL</div></div>						
	ORGANICS black SILT some clay, trace sand, brown, wet	1BUNDIST										
		2AUNDIST										
		2BUNDIST										
		3AUNDIST										
		3BUNDIST										
22.5 3.0	SILT CLAY grey, wet				22							
20.7					21							
4.9	END OF BOREHOLE 1. Sample refusal at 4.9 m. 2. Borehole backfilled with bentonite upon completion.											

GROUNDWATER ELEVATIONS
Shallow Single Installation: [Symbol] Deep Dual Installation: [Symbol]
GRAIN SIZE NOTES: + 3, x 3, Numbers refer to Sieve Size. ○ 5-3% Spinn at Failure

PROJECT: Mr. Christie	DRILLING DATA	REF. NO.: 1889-220
CLIENT: [REDACTED]	Method: Geo Probe	ENC. NO.:
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:	
DATE/LOC: Local	Date: Nov/15/2013	
BH LOCATION:		

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT	PLASTIC LIMIT w_p	NATURAL CONTENT w	LIQUID LIMIT w_L	POCKET PEN. (C_u) (kPa)	NATURAL UNIT WT (M_g/m^3)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)							
DEPTH (m)	DESCRIPTION	NUMBER	TYPE																
26.2	TOPSOIL	1AUNDIST			26		w_p	w	w_L										
25.9	FILL																		
25.9	silt, some sand, trace gravel, trace clay, trace slag, brown, moist																		
25.9																			
25.9																			
24.1	SILT	2AUNDIST			24														
23.9	organic silt																		
23.9	SILT	2BUNDIST																	
23.9	silt, some clay, trace sand, brown, moist																		
23.2	CLAYEY SILT	3AUNDIST			23														
3.0	clayey silt, brown, moist																		
21.6	SILT	3BUNDIST			22														
4.6	silt, some sand, trace gravel, brown, saturated																		
		4AUNDIST			21														
20.1	SILTY CLAY	4BUNDIST			20														
6.1	silty clay, grey, saturated																		
		5AUNDIST																	
		5BUNDIST			19														
18.3		6AUNDIST			18														
7.9																			
END OF BOREHOLE																			
1. Sample refusal at 7.9 m.																			
2. 50mm-diameter monitoring well installed at 8.4 m.																			

GROUNDWATER ELEVATIONS
Shallow Single Installation: [Symbol] Deep Dual Installation: [Symbol]
GRAIN SIZE NOTES: + 3, x 3, Numbers refer to Sieve Size. ○ 5-3% Spinn at Failure

PROJECT: M. Christie	DRILLING DATA
CLIENT: [REDACTED]	Method: Geo Probe
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON	Diameter:
DATUM: Local	Date: Nov/15/2013
BH LOCATION:	REF. NO.: 1889-220
	ENCL. NO.:

SOIL PROFILE		SAMPLES		REMARKS	
(m)	DESCRIPTION	NUMBER	TYPE	ELEVATION	POCKET PEN. (Coi (kPa))
25.8	ASPHALT				
25.4	FILL				
0.5	SILT				
	some clay, trace sand, trace gravel, brown, moist				
24.0	CLAYEY SILT				
1.8	grey, moist				
		1AUNDIST	"N" BLOWS 0.3 m	25	
		1BUNDIST		25	
		2AUNDIST		24	
		2BUNDIST		23	
		3AUNDIST		22	
		3BUNDIST		21	
		4AUNDIST		20	
		4BUNDIST			
		5AUNDIST			
19.6	END OF BOREHOLE				
6.2	1.5 m diameter, 6.2 m				
	2.50 mm-diameter monitoring well				
	installed at 0.7 m				

PROJECT: Mr. Chrishe
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE					SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION										PLASTIC LIMIT			POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" BLOWS 0.3 m			SHEAR STRENGTH (KPa) FIELD VALUE ● CLICK TRIAXIAL × LAB VALUE	W _p	NATURAL CONTENT W	LIQUID LIMIT W _L												
25.5		ASPHALT																						
25.2		FILL																						
25.2		125 mm of asphalt sand & gravel			1AUNDIST			25																
0.3		SAND																						
		sand, trace silt, brown/grey, wet			1BUNDIST																			
24.3		SILTY CLAY																						
1.2		grey, moist			2AUNDIST			24																
					2BUNDIST																			
23.0		END OF BOREHOLE																						
2.4		1. Sample refusal at 2.4 m. 2. Borehole backfilled with bentonite upon completion.																						

GROUNDWATER ELEVATIONS
Shallow Single Installation Deep Dual Installation

GRAPH NOTES
+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 7.GPJ SPL.GDT 12/5/13

PROJECT: Mr. Chrishe
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/14/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE			SAMPLES		GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT	PLASTIC LIMIT w_p	NATURAL CONTENT w	LIQUID LIMIT w_L	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (Mg/m^3)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
(m)	ELEV DEPTH	DESCRIPTION	NUMBER	TYPE									
25.9		TOPSOIL											
25.9		FILL											
0.2		dark brown silt, some clay, trace sand, brown, moist		1AUNDIST									
24.8		SILT											
1.1		some clay		1BUNDIST		25							
				2AUNDIST									
				2BUNDIST		24							
23.5		grey, wet											
2.4		END OF BOREHOLE											
		1. Sample refusal at 2.4 m. 2. Borehole backfilled with bentonite upon completion.											

GROUNDWATER ELEVATIONS
Shallow Single Installation Deep Dual Installation

GRAPH NOTES
+ 3, - 3: Numbers refer to Sensitivity
○ 5-3% Span at Failure

1 OF 1

[illegible]

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/15/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS		DYNAMIC CONE PENETRATION		PLASTIC MOISTURE		POCKET PEN.		REMARKS	
(m)	ELEV. / DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS @ 0.3 N	ELEVATION	SHEAR STRENGTH (kPa)	FIELD VANE	WATER CONTENT (%)	POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
25.5		ASPHALT											
24.9		75 mm of asphalt											
24.9		FILL sand and gravel, brown, moist		1AUNDIST			25						
0.6		SILTY CLAY organics, black, moist											
				18JUNDIST			24						
				2AUNDIST			23						
				28JUNDIST									
22.5		CLAYEY SILT brown					22						
3.0				3AUNDIST									
		grey, wet		38JUNDIST			21						
20.9													
4.6		END OF BOREHOLE											
		1. Sample refusal at 2.4 m.											
		2. Borehole backfilled with bentonite upon completion.											

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 8.GPJ SPL.GDT 12/5/13

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:

GRAPH NOTES
+ 3, - 3: Numbers refer to Sensitivity
○ 5-3%: Span at Failure

PROJECT: Mr. Christie
CLIENT: XXXXXXXXXX
PROJECT LOCATION: 2150 Lake Shore Blvd, Toronto, ON
DATUM: Local
BH LOCATION:

DRILLING DATA
Method: Geo Probe
Diameter:
Date: Nov/15/2013

REF. NO.: 1889-220
ENCL. NO.:

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS		DYNAMIC CONE PENETRATION		PLASTIC MOISTURE		POCKET PEN.		REMARKS	
(m)	ELEV. / DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	BLOWS @ 0.3 N	ELEVATION	SHEAR STRENGTH (kPa)	FIELD VANE	WATER CONTENT (%)	POCKET PEN. (Cu) (MPa)	NATURAL UNIT WT (Mg/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
25.6		ASPHALT											
24.8		75 mm of asphalt											
		SILT some clay, trace sand, trace gravel, grey/orange, moist		1AUNDIST			25						
				18JUNDIST			24						
				28JUNDIST			23						
23.8		CLAYEY SILT grey, moist					22						
1.8				2AUNDIST									
				3AUNDIST									
				38JUNDIST			21						
		saturated		4AUNDIST									
20.2													
5.5		SILTY CLAY grey, wet		48JUNDIST			20						
19.5													
6.2		END OF BOREHOLE											
		1. Sample refusal at 1.6 m.											
		2. Borehole backfilled with bentonite upon completion.											

SPL SOIL LOG 1889-220 BOREHOLE LOGS SET 8.GPJ SPL.GDT 12/5/13

GROUNDWATER ELEVATIONS
Shallow Single Installation: Deep Dual Installation:

GRAPH NOTES
+ 3, - 3: Numbers refer to Sensitivity
○ 5-3%: Span at Failure



GEO-CANADA

LOG OF BOREHOLE 1

1 OF 1

CLIENT: Kraft Canada Inc.
PROJECT: Transformer Upgrades
LOCATION: Toronto

DRILLING DATA
Method: Continuous flight solid-stem augers
Diameter: 110mm

REF. NO.: G-04-0903
ENCL. NO.: 1

DATUM ELEVATION: Assumed Datum - See Drawing No. 1 for Location, October 15, 2004

BOREHOLE LOCATION: Refer to Drawing No. 1

Geo-Canada Soil Log

Borehole Logs G040903.GPJ

Geo-Canada Template GDT 2/11/04

BOREHOLE LOCATION: Refer to Drawing No. 1

SOIL PROFILE

(m)
ELEV
DEPTH

DESCRIPTION

101.5 Ground Surface

TOPSOIL 25mm

0.0 Silt, trace sand, trace gravel and clay, brown, stiff (possible FILL)

100.9 CLAYEY SILT

0.6 mottled brown-grey, stiff to very stiff

.....

grey

STRATA PLOT

NUMBER

TYPE

SS 10

SS 14

SS 21

SS 18

SS 17

SS 32

SS 12

SS 60

SS 60

SS 60

GROUND WATER CONDITIONS

ELEVATION

101

100

99

98

97

96

95

94

93

DYNAMIC CONE PENETRATION

RESISTANCE PLOT

20 40 60 80 100

SHEAR STRENGTH (kPa)

0 UNCONFINED + FIELD VANE

20 40 60 80 100

QUICK TRIAXIAL x LAB VANE

NATURAL MOISTURE CONTENT (%)

10 20 30

W_p

W_L

LIT WEIGHT

γ

(kN/m³)

REMARKS AND GRAIN SIZE DISTRIBUTION (%)

GR SA SI CL

END OF BOREHOLE

Borehole dry upon completion.

Water Level Readings:

Date

Oct. 22/04

3.25

8 73 19

7 89 25

Spill spoon well.

GRAPEH +3 X 3: Numbers refer to Sensitivity O 6-3% Spun at Failure

GRAPEH +3 X 3: Numbers refer to Sensitivity O 6-3% Spun at Failure



GEO-CANADA

LOG OF BOREHOLE 2

1 OF 1

CLIENT: Kraft Canada Inc.
PROJECT: Transformer Upgrades
LOCATION: Toronto

DRILLING DATA
Method: Continuous flight solid-stem augers
Diameter: 110mm

REF. NO.: G-04-0903
ENCL. NO.: 2

DATUM ELEVATION: Assumed Datum - See Drawing No. 1 for Location, October 15, 2004

BOREHOLE LOCATION: Refer to Drawing No. 1

101.7

Ground Surface

0.0

TOPSOIL 2.0mm

Silt, some clay, organic stained, reddish, brown (Possible FILL)

101.1

CLAYEY SILT

0.6

mottled brown grey, occ. fissures very stiff to hard

98.7

SILT

3.0

some clay, occ. very thin silt partings grey, very stiff

very moist

clayey

95.6

CLAY

6.1

some silt, interlayered with silt, some clay, stiff, very moist

94.4

BEDROCK

7.3

SHALE (interbed)

grey

92.6

END OF BOREHOLE

9.1

Borehole dry upon completion. Water Level Readings: Date W.L. Depth (m) Oct 22/04 5.45

101

1

SS

16

2

SS

18

3

SS

25

4

SS

32

5

SS

22

6

SS

17

7

SS

9

8

SS

60

9

SS

60

0

GRAB

101

100

99

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80

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GRAPEH +3 X 3: Numbers refer to Sensitivity O 6-3% Spun at Failure

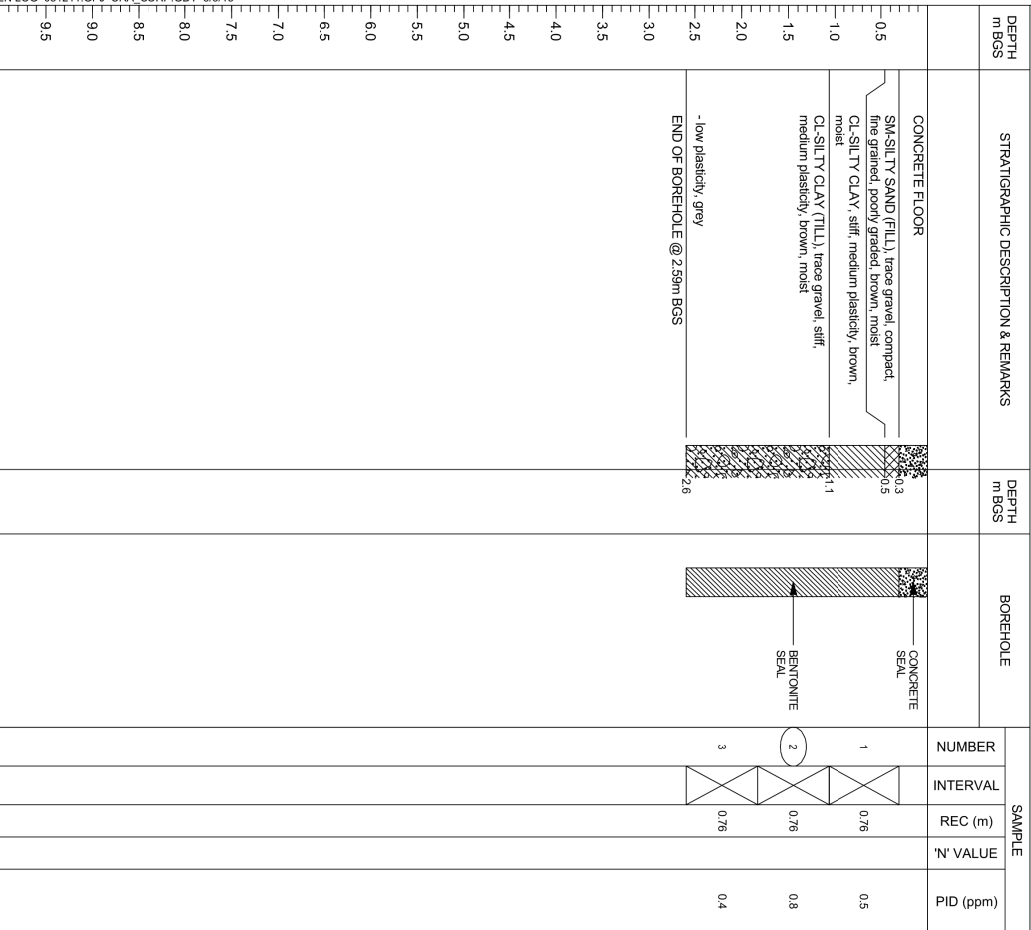


STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH1-13
DATE COMPLETED: March 2, 2013
DRILLING METHOD: SPLIT-SPOON
FIELD PERSONNEL: K. Vander Meulen



NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

LABORATORY ANALYSIS

OVERBURDEN LOG 081211.GPJ CRA_CORP.GDT 3/3/13

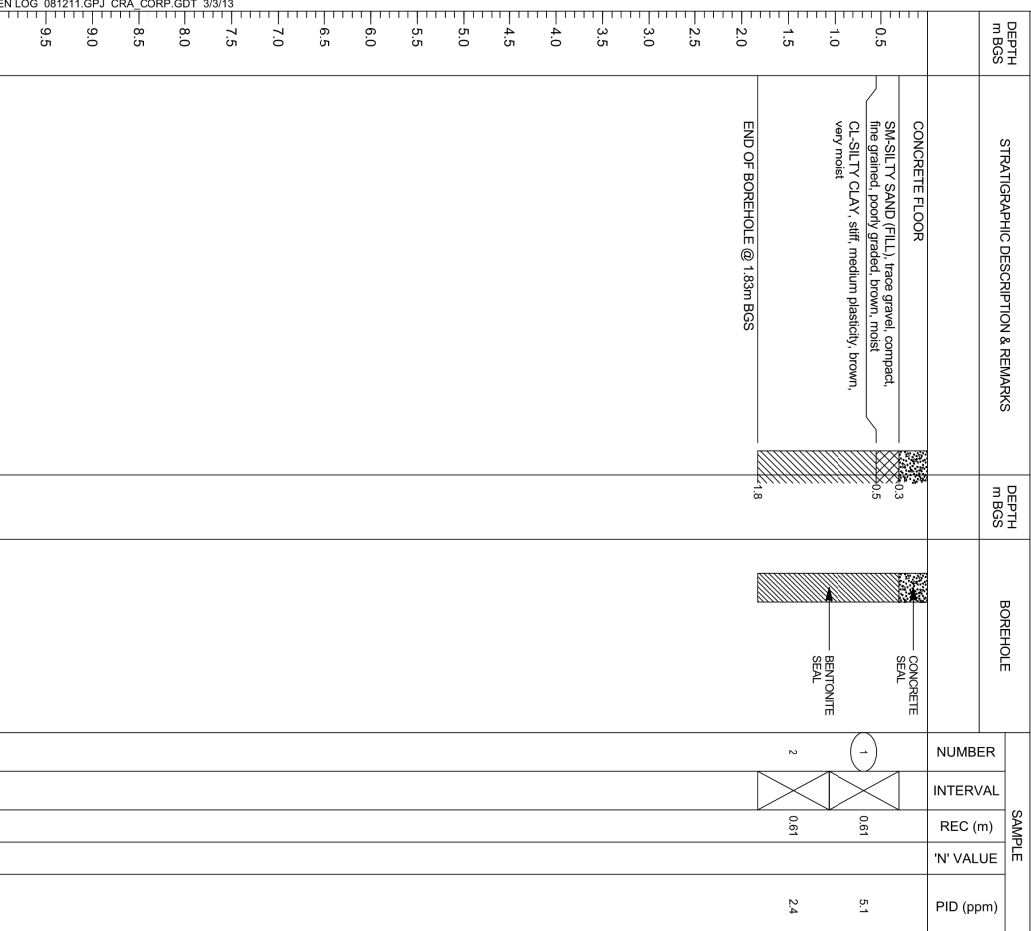


STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH2-13
DATE COMPLETED: March 2, 2013
DRILLING METHOD: SPLIT-SPOON
FIELD PERSONNEL: K. Vander Meulen



NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

LABORATORY ANALYSIS

OVERBURDEN LOG 081211.GPJ CRA_CORP.GDT 3/3/13



STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH3-13
DATE COMPLETED: March 2, 2013
DRILLING METHOD: SPLIT-SPOON
FIELD PERSONNEL: K. Vander Meulen

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (m)	'N' VALUE
							PID (ppm)
0.5	CONCRETE FLOOR	0.2					
0.5	CL-SILTY CLAY, firm, medium plasticity, grey, moist			1		0.76	0.8
1.0	- very moist to wet			2		0.61	0.8
1.5	END OF BOREHOLE @ 1.52m BGS	1.5					
2.0							
2.5							
3.0							
3.5							
4.0							
4.5							
5.0							
5.5							
6.0							
6.5							
7.0							
7.5							
8.0							
8.5							
9.0							
9.5							

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

LABORATORY ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH4-13
DATE COMPLETED: March 2, 2013
DRILLING METHOD: SPLIT-SPOON
FIELD PERSONNEL: K. Vander Meulen

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (m)	'N' VALUE
							PID (ppm)
0.5	CONCRETE FLOOR	0.2					
0.5	SW-GW SAND & GRAVEL (FILL) fine to coarse grained, well graded, brown, moist			1		0.76	0.4
1.0	- split-spoon refusal	0.9					
1.5	END OF BOREHOLE @ 0.91m BGS						
2.0							
2.5							
3.0							
3.5							
4.0							
4.5							
5.0							
5.5							
6.0							
6.5							
7.0							
7.5							
8.0							
8.5							
9.0							
9.5							

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE



STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH5-13
DATE COMPLETED: March 2, 2013
DRILLING METHOD: SPLIT-SPOON
FIELD PERSONNEL: K. Vander Meulen

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (m)	'N' VALUE
0.5	CONCRETE FLOOR	0.2	CONCRETE SEAL				
0.5	SW/GW SAND & GRAVEL (FILL)	0.3					
	END OF BOREHOLE @ 0.30m BGS						
1.0							
1.5							
2.0							
2.5							
3.0							
3.5							
4.0							
4.5							
5.0							
5.5							
6.0							
6.5							
7.0							
7.5							
8.0							
8.5							
9.0							
9.5							

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE



STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH6-13
DATE COMPLETED: March 2, 2013
DRILLING METHOD: SPLIT-SPOON
FIELD PERSONNEL: K. Vander Meulen

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (m)	'N' VALUE
0.5	CONCRETE FLOOR	0.2	CONCRETE SEAL				
0.5	SP SAND (FILL), fine grained, poorly graded, brown, moist	0.9		1	0.76	0.1	
1.0	CL-SILTY CLAY, firm, low plasticity, grey, moist		BENTONITE SEAL	2	0.76	0.2	
1.5	- stiff, brown, moist			3	0.61	0.1	
2.5	END OF BOREHOLE @ 2.44m BGS	2.4					
3.0							
3.5							
4.0							
4.5							
5.0							
5.5							
6.0							
6.5							
7.0							
7.5							
8.0							
8.5							
9.0							
9.5							

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

LABORATORY ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH101-13
DATE COMPLETED: February 7, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (m)	'N' VALUE PID (ppm)
	ASPHALT	0.2		1	P/S	0.61	0.1
	SW-GW SAND & GRAVEL (FILL) coarse to medium grained, well graded, brown, moist	0.9		2	P/S	0.61	0.9
	CL-SILTY CLAY, low plasticity, grey, moist			3	P/S	0.61	3.3
				4	P/S	0.61	9.3
				5	P/S	0.61	1.3
	- medium plasticity			6	P/S	0.61	199
	- brown, moist to wet			7	P/S	0.61	0.6
				8	P/S	0.61	0.1
				9	P/S	0.61	0.0
				10	P/S	0.61	0.0
	END OF BOREHOLE @ 6.10m BGS	6.1					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

LABORATORY ANALYSIS

OVERBURDEN LOG 081211.GPJ CRA_CORP.GDT 2/25/13



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH102-13
DATE COMPLETED: February 7, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (m)	'N' VALUE PID (ppm)
	ASPHALT	0.2		1	P/S	0.61	1.5
	SW-GW SAND & GRAVEL (FILL) well graded, grey, moist	0.6		2	P/S	0.61	1.6
	SP-SAND (FILL), medium grained, poorly graded, brown, moist			3	P/S	0.61	5.2
				4	P/S	0.61	0.3
				5	P/S	0.61	10.3
	- moist to wet, slight black staining, slight odour	3.0		6	P/S	0.61	283
	- 152 mm section of coarse gravel, grey, slight odour	4.0		7	P/S	0.61	1.4
	CL-SILTY CLAY, medium plasticity, grey, moist	4.3		8	P/S	0.61	21.9
	SM-SILTY SAND, fine grained, poorly graded, brown, wet	4.3		9	P/S	0.61	1.2
	CL-SILTY CLAY, medium plasticity, grey, moist			10	P/S	0.61	1.4
	- less silt content, high plasticity, grey, moist to wet						
	END OF BOREHOLE @ 6.10m BGS	6.1					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

LABORATORY ANALYSIS

OVERBURDEN LOG 081211.GPJ CRA_CORP.GDT 2/25/13

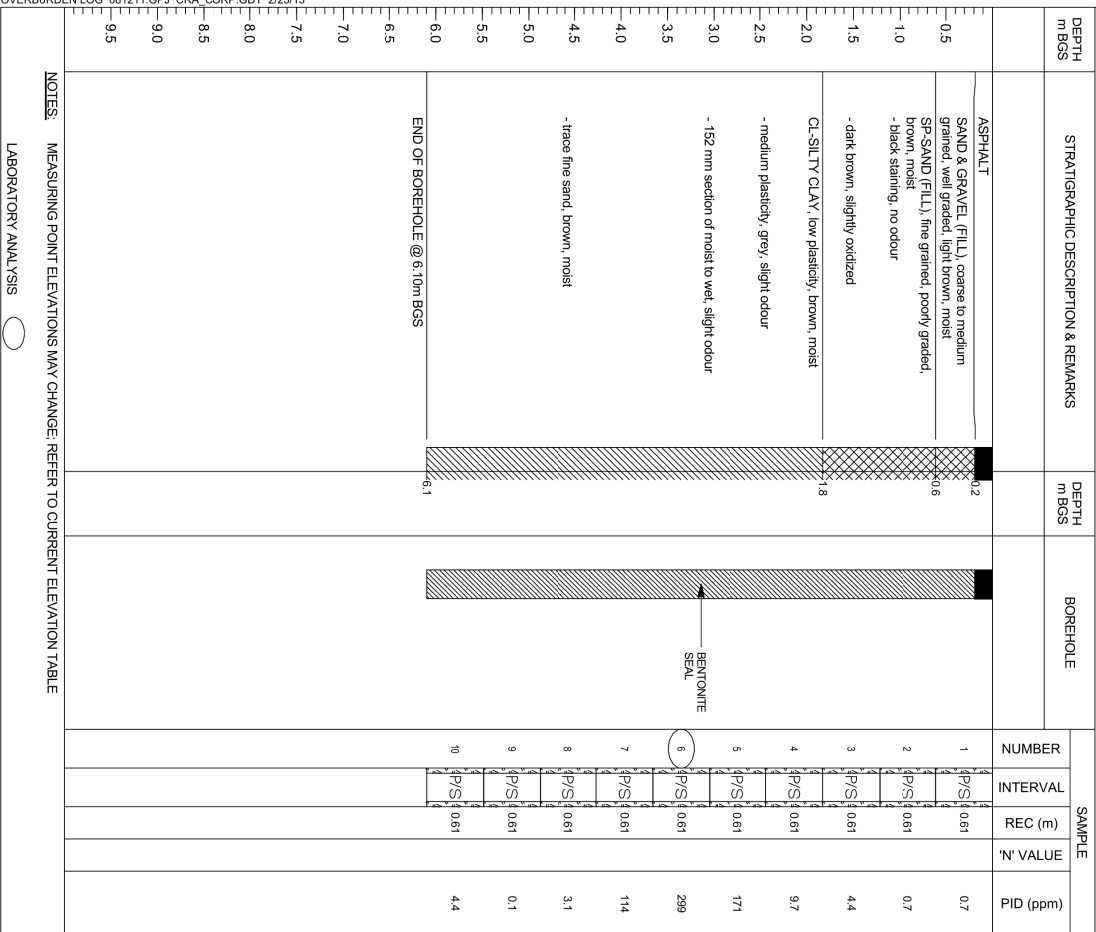


STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH103-13
DATE COMPLETED: February 7, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith

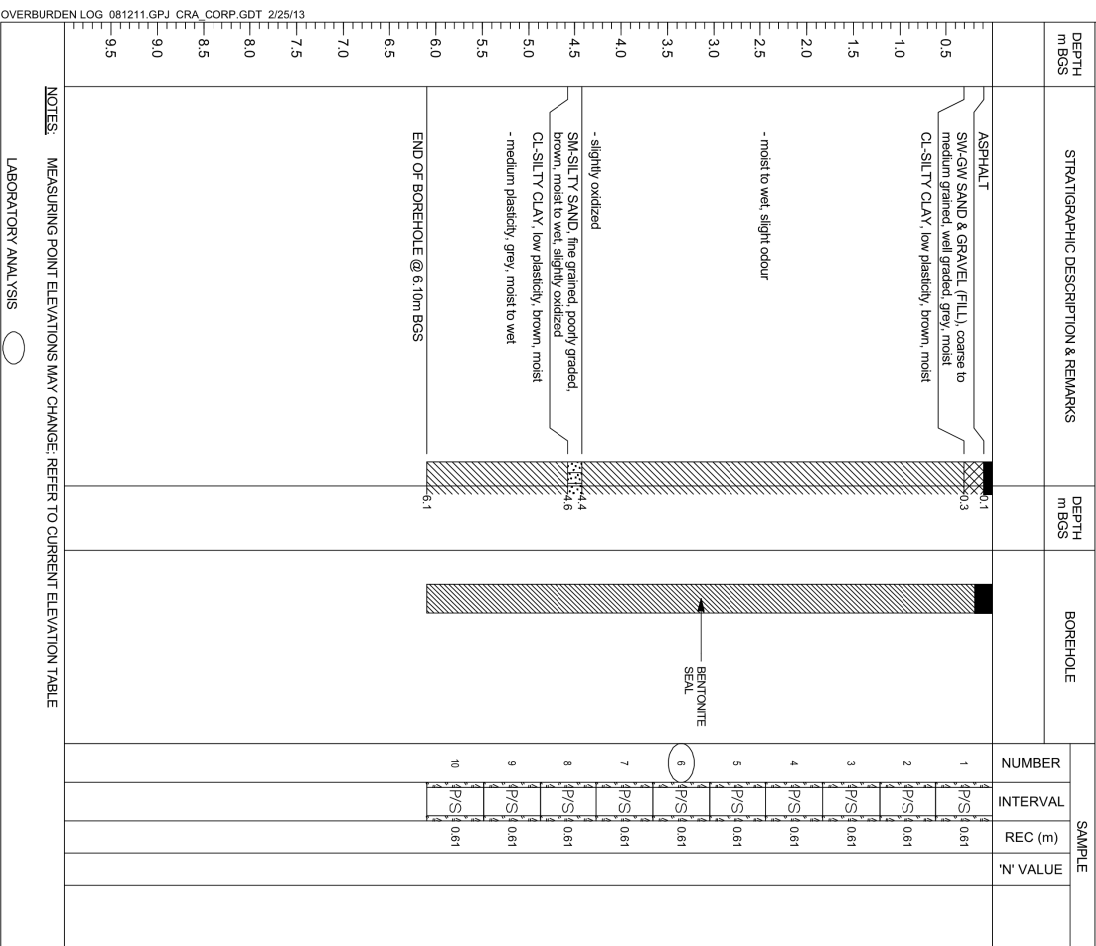


STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH104-13
DATE COMPLETED: February 7, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith





STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH105-13
DATE COMPLETED: February 7, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (m)	'N' VALUE
0.5	ASPHALT	0.2		1	P/S	0.61	
0.5	SW-GW SAND & GRAVEL (FILL), coarse to medium grained, well graded, grey, moist	0.3		2	P/S	0.61	
1.0	SP-SAND (FILL), medium grained, poorly graded, brown, moist	0.6		3	P/S	0.61	
1.5	SW-SILTY SAND (FILL), medium grained, poorly graded, moist, oxidized	0.8		4	P/S	0.61	
2.0	CL-SILTY CLAY, low plasticity, brown, moist			5	P/S	0.61	
2.5	- slight black staining, no odour			6	P/S	0.61	
3.0	- brown / grey, slightly oxidized, slight odour			7	P/S	0.61	
3.5	- moist to wet			8	P/S	0.61	
4.0	- slightly oxidized			9	P/S	0.61	
4.5	- slightly oxidized			10	P/S	0.61	
5.0	- slightly oxidized						
5.5	- medium plasticity, grey, moist to wet						
6.0	END OF BOREHOLE @ 6.10m BGS	6.1					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

LABORATORY ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH106-13
DATE COMPLETED: February 7, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (m)	'N' VALUE
0.5	ASPHALT	0.1		1	P/S	0.61	1.8
0.5	SW-GW SAND & GRAVEL (FILL), medium to fine grained, well graded, brown, moist	0.3		2	P/S	0.61	0.9
1.0	SP-SAND (FILL), fine grained, poorly graded, brown, moist	0.5		3	P/S	0.61	6.5
1.5	CL-SILTY CLAY, low plasticity, grey, moist, slightly oxidized	1.1		4	P/S	0.61	105
2.0	- slight odour, grey			5	P/S	0.61	82.3
2.5	- strong odour, wet			6	P/S	0.61	203
3.0	- strong odour, moist			7	P/S	0.61	0.8
3.5	- slight odour, brown			8	P/S	0.61	1.4
4.0	- grey			9	P/S	0.61	0.6
4.5	- brown			10	P/S	0.61	0.2
5.0	- 152 mm section of fine silty sand, brown, wet						
5.5	- grey, moist						
6.0	END OF BOREHOLE @ 6.10m BGS	6.1					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

LABORATORY ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA

HOLE DESIGNATION: BH107-13

PROJECT NUMBER: 081211

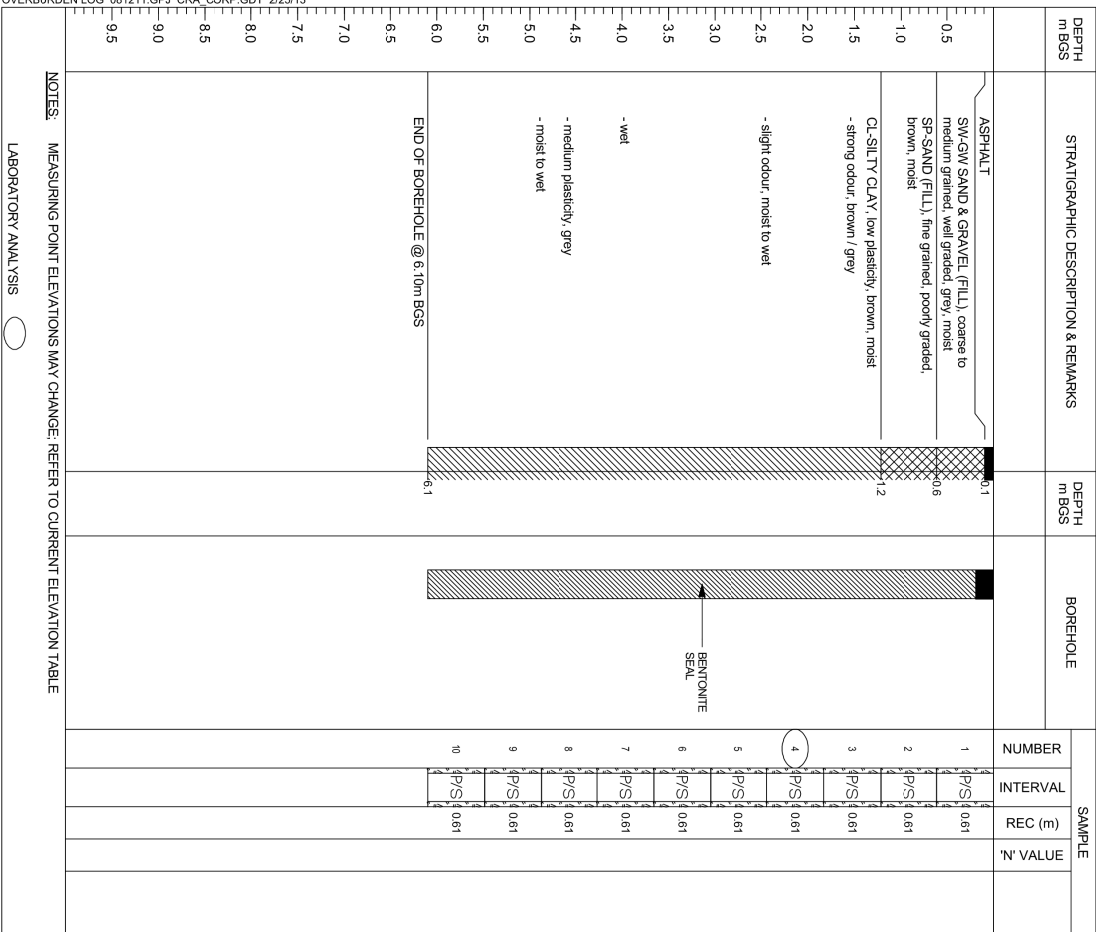
DATE COMPLETED: February 7, 2013

CLIENT: Mondelēz Canada Inc.

DILLING METHOD: GEOPROBE

LOCATION: 2150 Lake Shore Blvd. West, Toronto

FIELD PERSONNEL: L. Griffith



STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA

HOLE DESIGNATION: BH108-13

PROJECT NUMBER: 081211

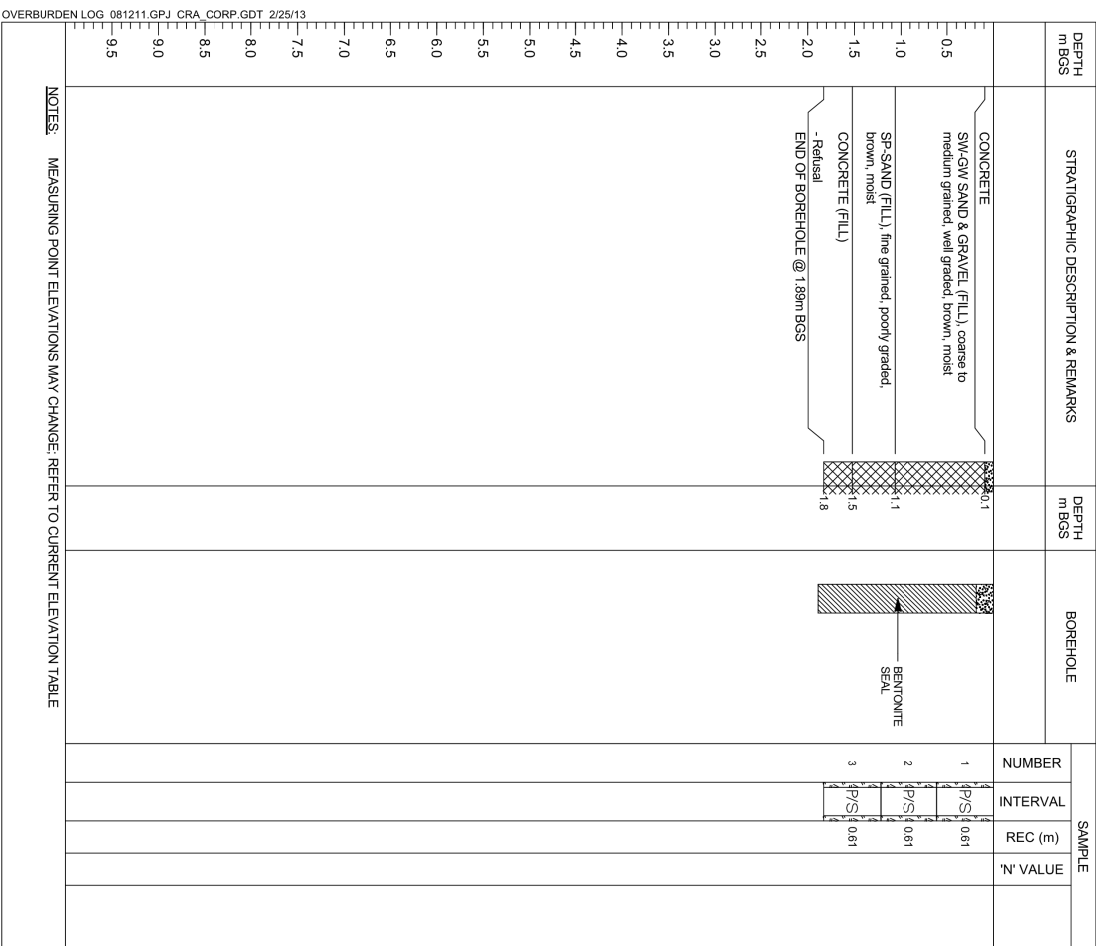
DATE COMPLETED: February 7, 2013

CLIENT: Mondelēz Canada Inc.

DILLING METHOD: GEOPROBE

LOCATION: 2150 Lake Shore Blvd. West, Toronto

FIELD PERSONNEL: L. Griffith





STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA

HOLE DESIGNATION: BH109-13

PROJECT NUMBER: 081211

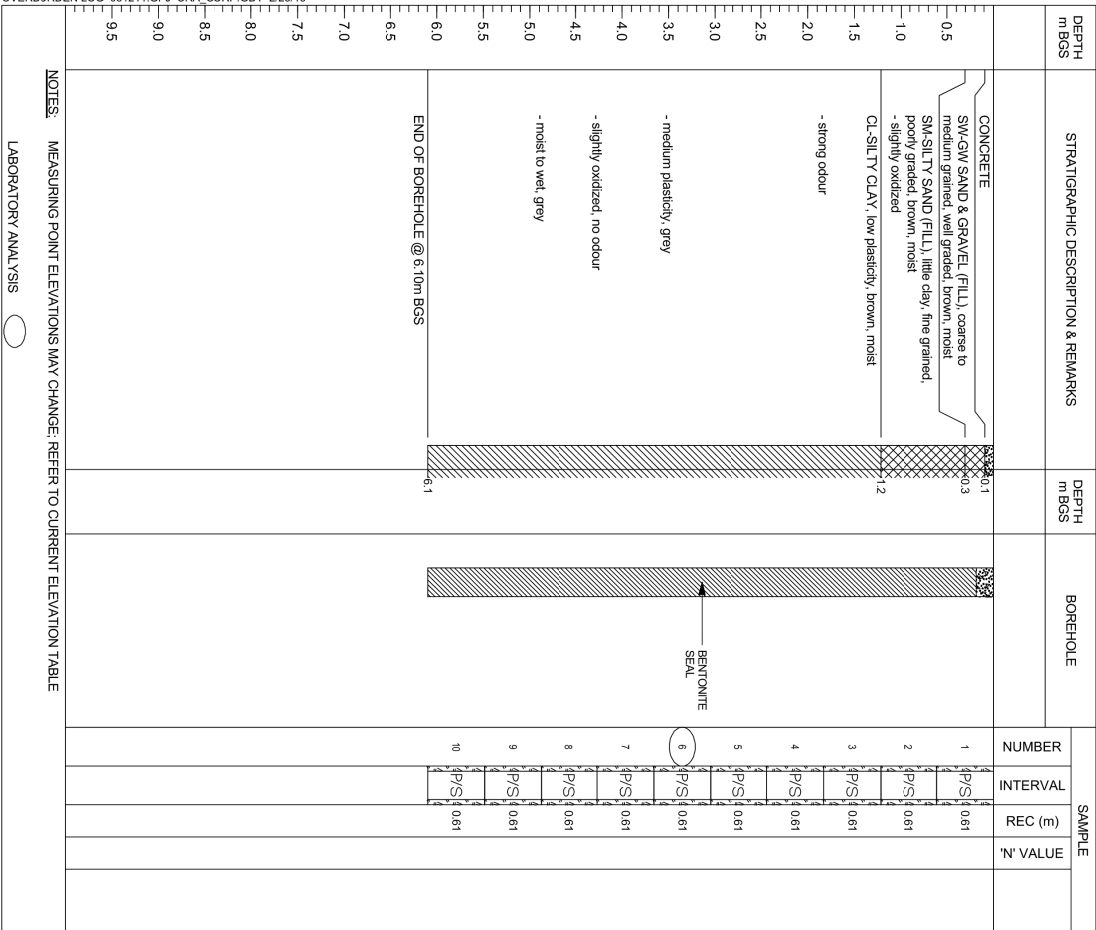
DATE COMPLETED: February 7, 2013

CLIENT: Mondelēz Canada Inc.

DILLING METHOD: GEOPROBE

LOCATION: 2150 Lake Shore Blvd. West, Toronto

FIELD PERSONNEL: L. Griffith



STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA

HOLE DESIGNATION: BH110-13

PROJECT NUMBER: 081211

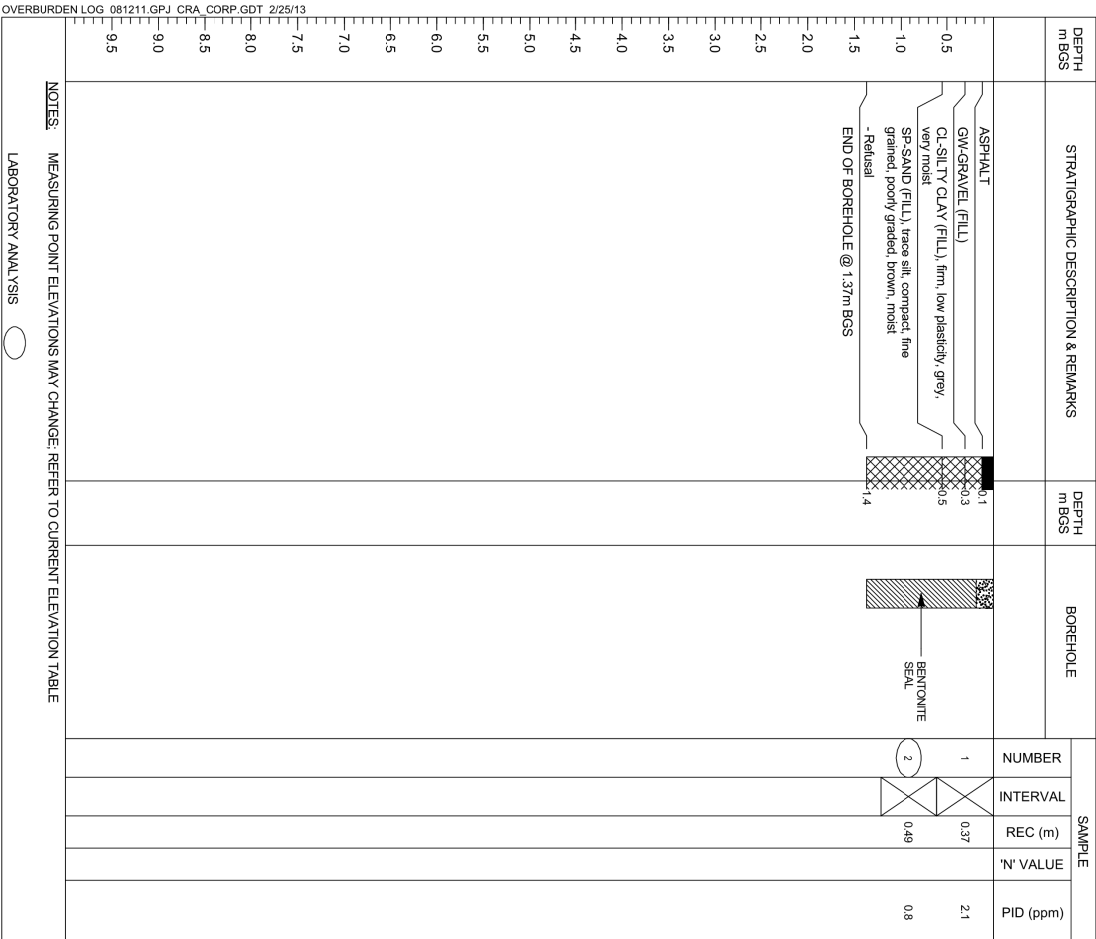
DATE COMPLETED: February 22, 2013

CLIENT: Mondelēz Canada Inc.

DILLING METHOD: HAND DRILL

LOCATION: 2150 Lake Shore Blvd. West, Toronto

FIELD PERSONNEL: L. Griffith



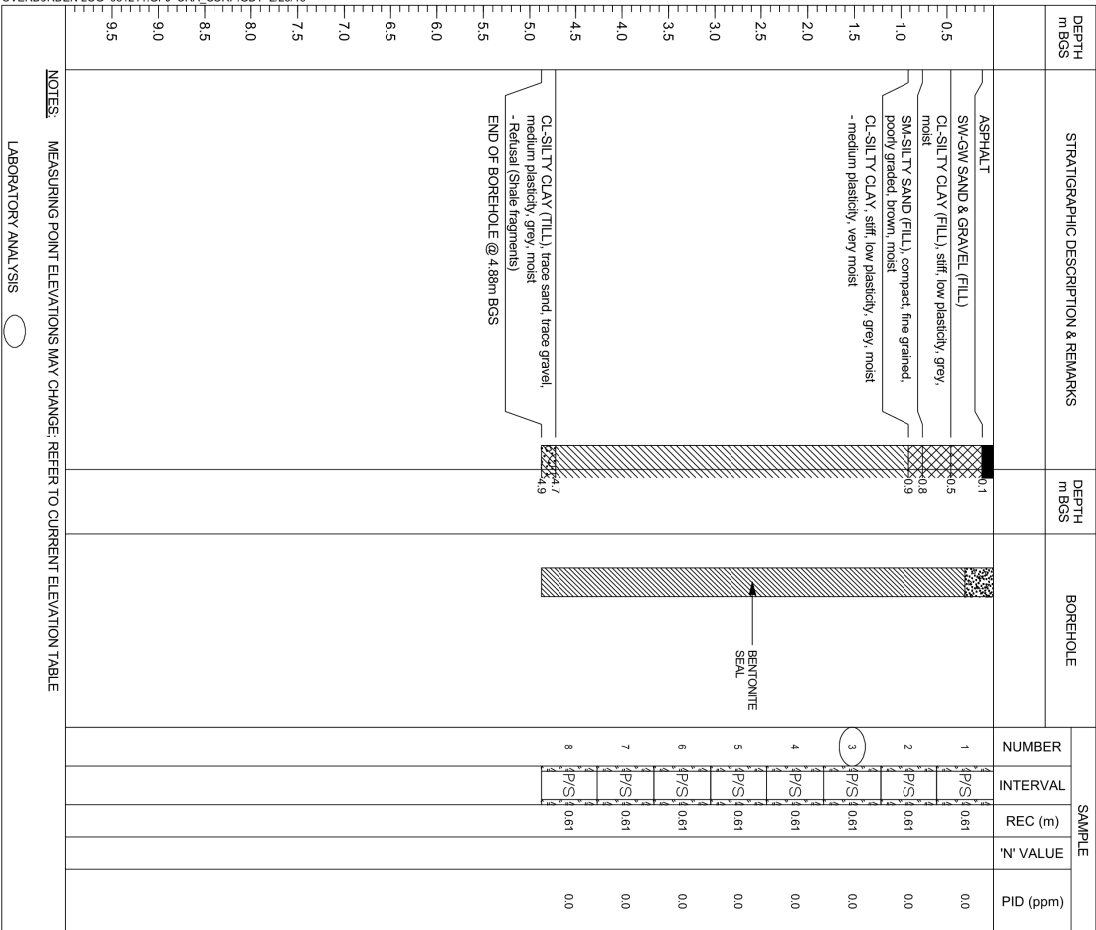


STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH111-13
DATE COMPLETED: February 22, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith

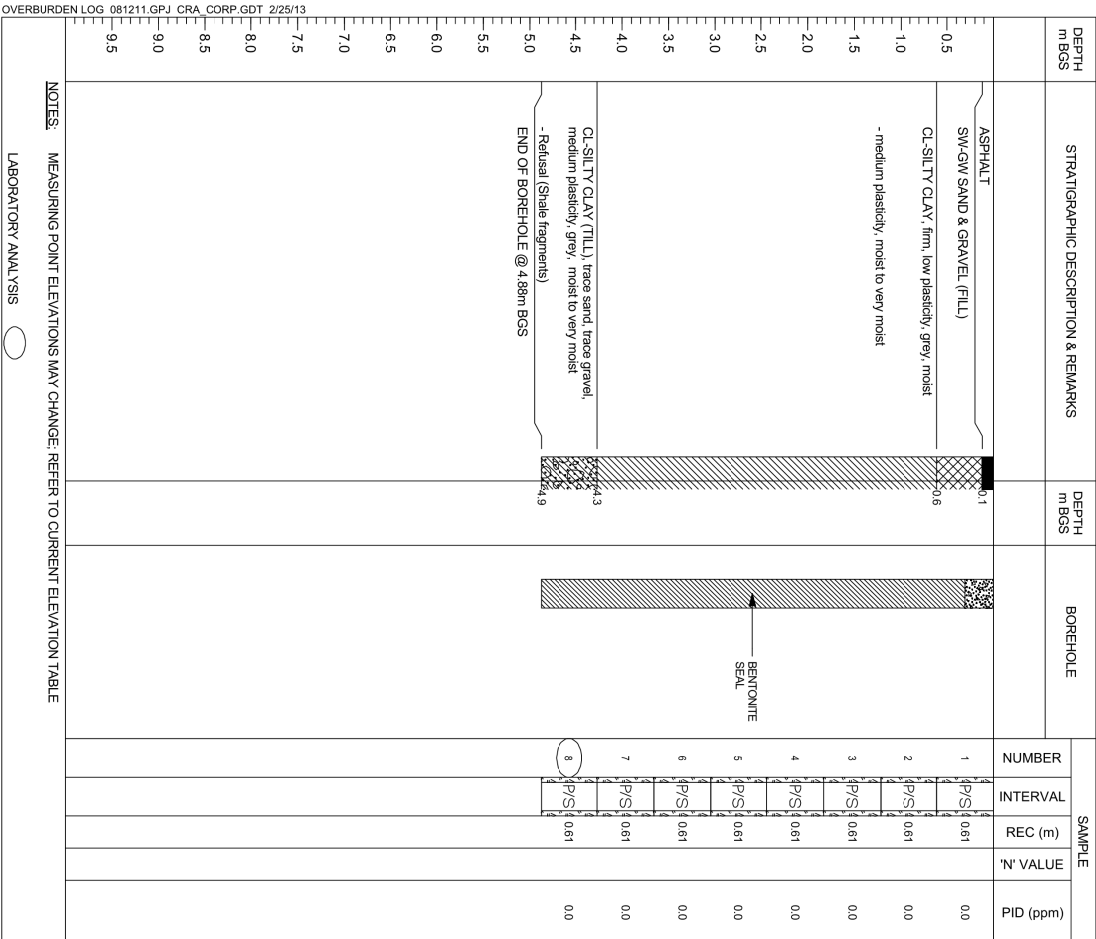


STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH112-13
DATE COMPLETED: February 22, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith



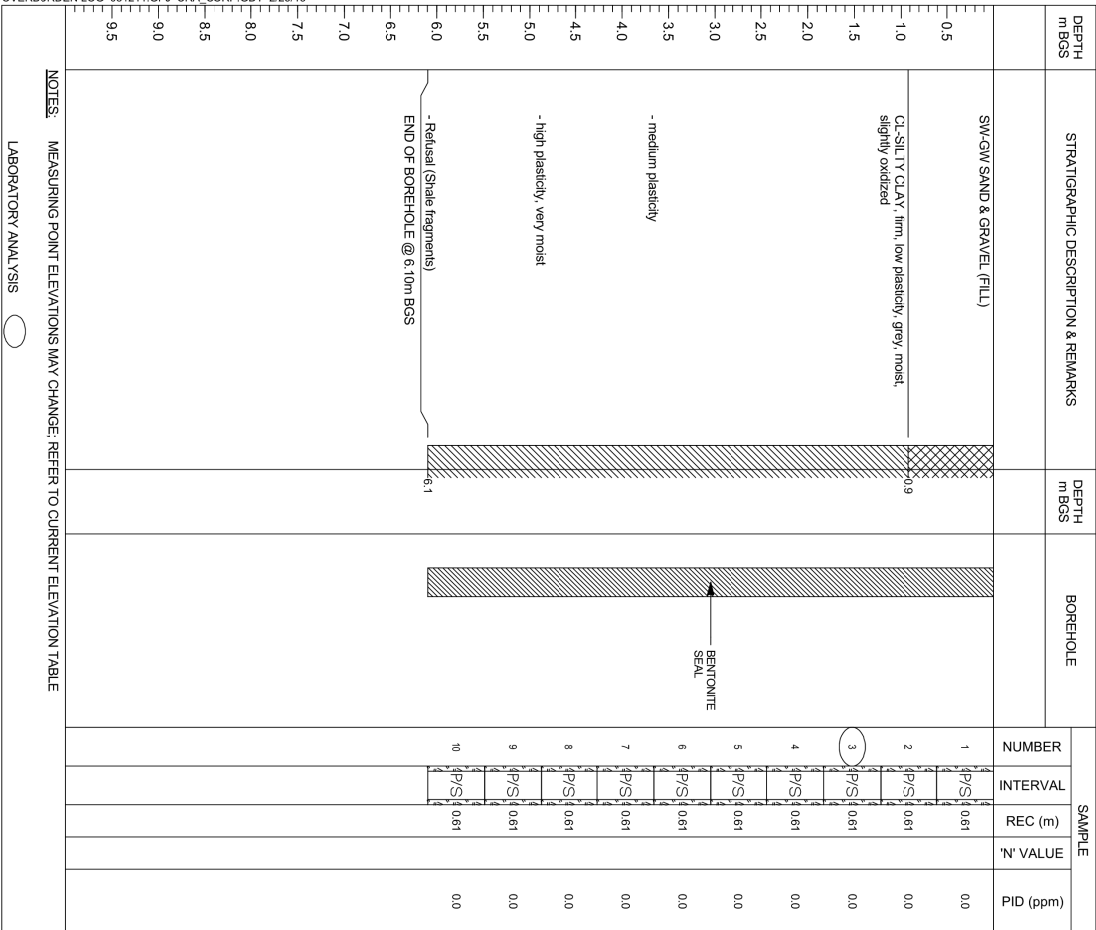


STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH113-13
DATE COMPLETED: February 22, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith

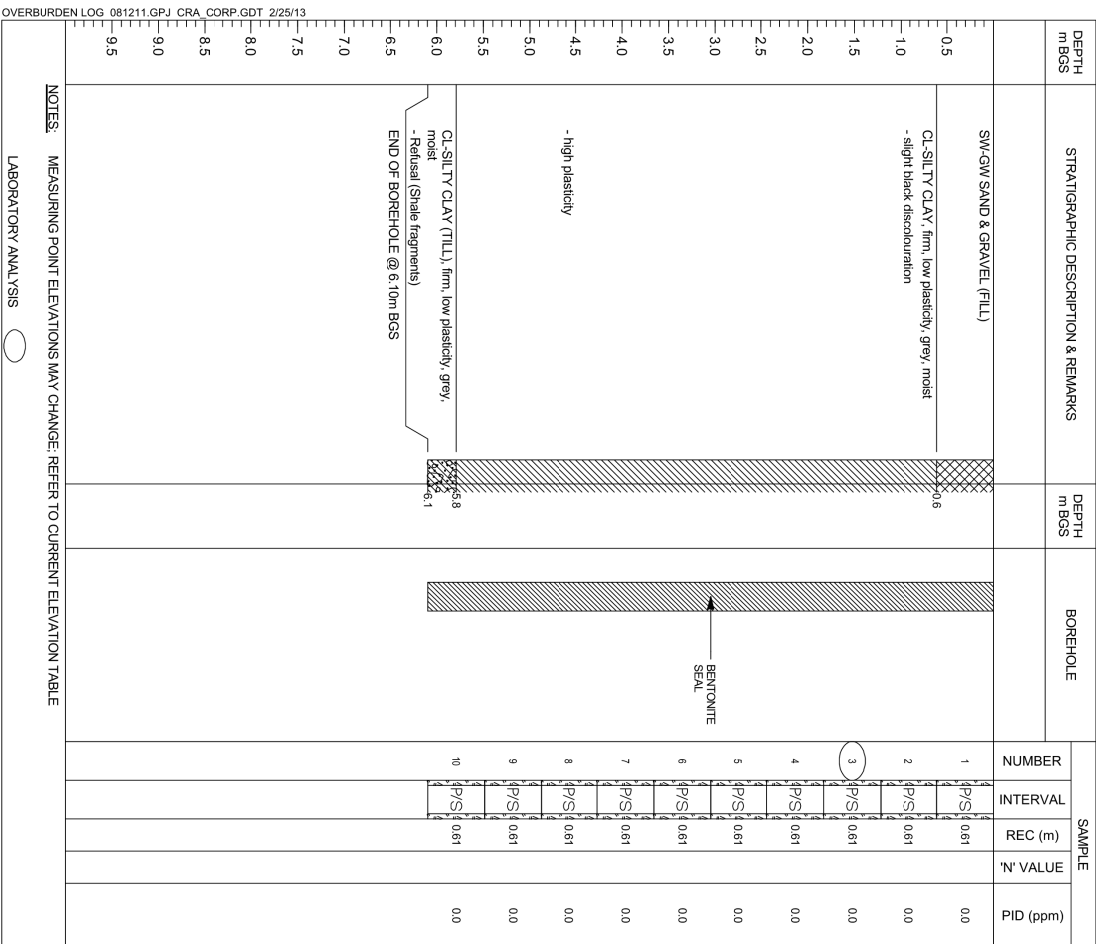


STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: Geotechnical Investigation and Phase Two ESA
PROJECT NUMBER: 081211
CLIENT: Mondelēz Canada Inc.
LOCATION: 2150 Lake Shore Blvd. West, Toronto

HOLE DESIGNATION: BH114-13
DATE COMPLETED: February 22, 2013
DRILLING METHOD: GEOPROBE
FIELD PERSONNEL: L. Griffith





STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: MONDELEZ
PROJECT NUMBER: 081211

CLIENT:

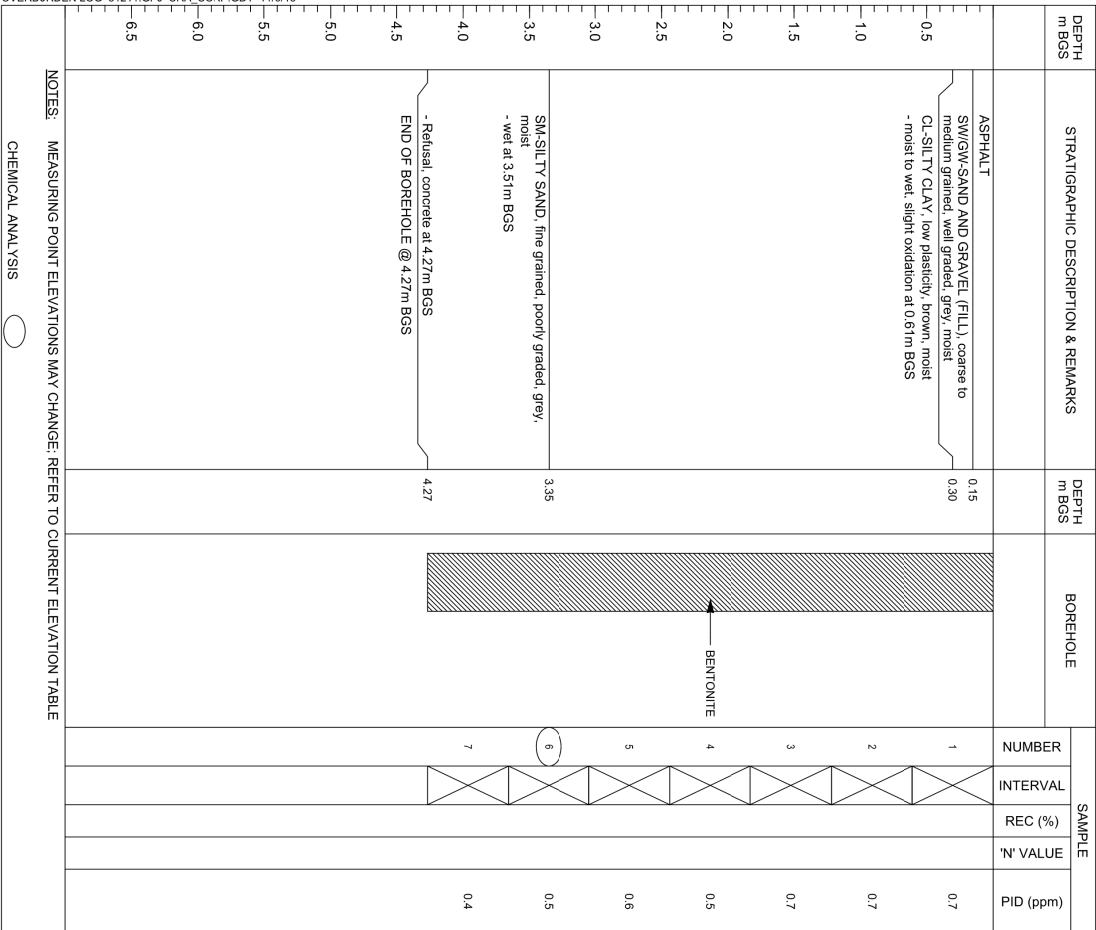
LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH201-13

DATE COMPLETED: 27 May 2013

DILLING METHOD: 51mm O.D./DIRECT PUSH

FIELD PERSONNEL: L. GRIFFITH



STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: MONDELEZ
PROJECT NUMBER: 081211

CLIENT:

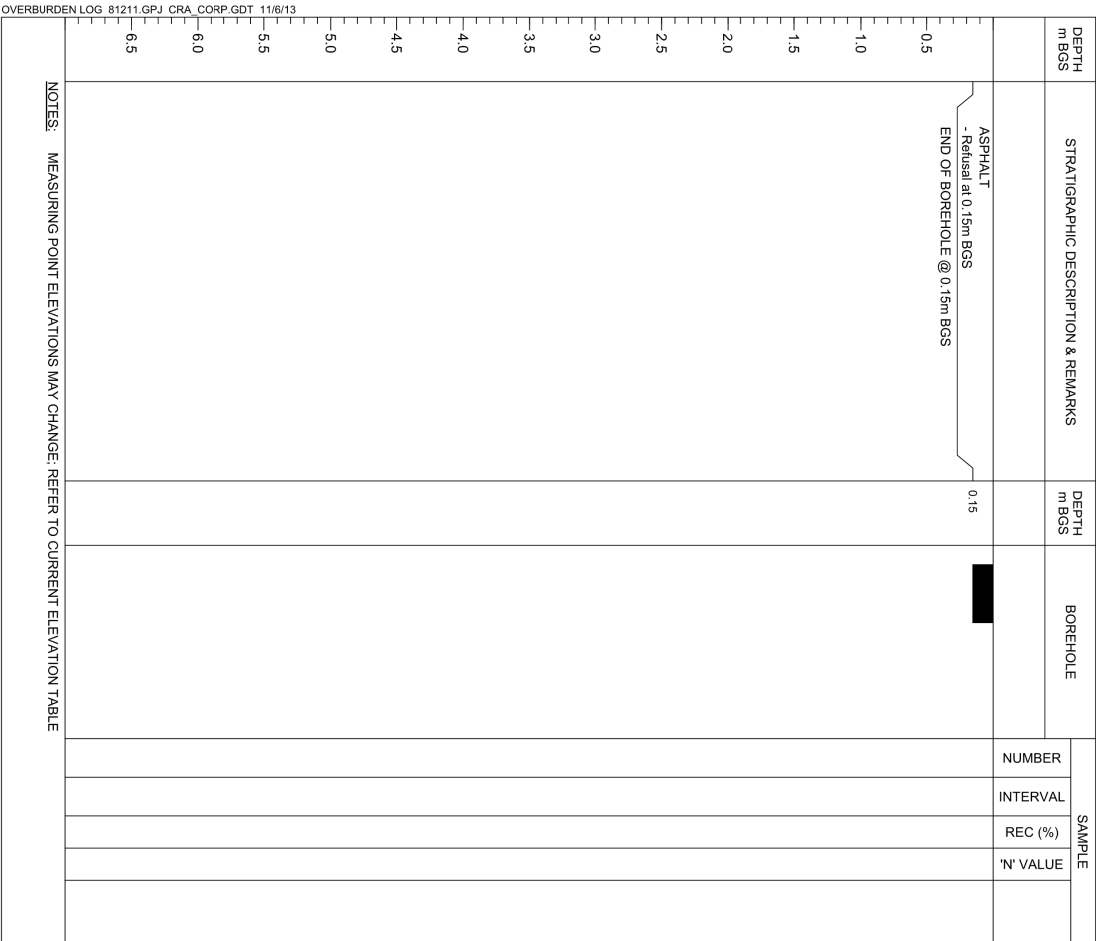
LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH202-13

DATE COMPLETED: 27 May 2013

DILLING METHOD: 51mm O.D./DIRECT PUSH

FIELD PERSONNEL: L. GRIFFITH





STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

Page 1 of 1

PROJECT NAME: MONDELEZ
PROJECT NUMBER: 081211

HOLE DESIGNATION: BH203-13
DATE COMPLETED: 27 May 2013

CLIENT:

DILLING METHOD: 51mm O.D./DIRECT PUSH

LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

FIELD PERSONNEL: L. GRIFFITH

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (%)	'N' VALUE
0.5	ASPHALT - Refusal at 0.15m BGS END OF BOREHOLE @ 0.15m BGS	0.15					
1.0							
1.5							
2.0							
2.5							
3.0							
3.5							
4.0							
4.5							
5.0							
5.5							
6.0							
6.5							

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

OVERBURDEN LOG 81211.GPJ CRA CORP.GDT 11/6/13



STRATIGRAPHIC AND INSTRUMENTATION LOG
(OVERBURDEN)

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PROJECT NAME: MONDELEZ
PROJECT NUMBER: 081211

HOLE DESIGNATION: BH204-13
DATE COMPLETED: 27 May 2013

CLIENT:

DILLING METHOD: 51mm O.D./DIRECT PUSH

LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

FIELD PERSONNEL: L. GRIFFITH

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (%)	'N' VALUE
0.5	ASPHALT SW/GW SAND AND GRAVEL (FILL) coarse to medium grained, well graded, brown, moist	0.15		1			0.8
0.5	SP-SAND (FILL), fine grained, poorly graded, brown, moist	0.61		2			0.9
1.0	SM-SILTY SAND, fine grained, poorly graded, brown, moist	0.91		3			0.7
1.5	ML-SILT, trace clay, low plasticity, brown, moist	1.22		4			0.9
2.0	- little sand, wet, slight oxidation at 1.83m BGS			5			2.1
2.5				6			2.4
3.0				7			0.7
3.5	SP-SAND, fine grained, poorly graded, brown, moist	3.66		8			1.0
4.0	ML-SILT, trace clay, low plasticity, brown, moist, slight oxidation	3.96		9			0.7
4.5	CL-SILTY CLAY, low plasticity, grey, moist to wet - trace sand at 4.27m BGS	4.11		10			0.6
5.0	- low plasticity, moist at 4.57m BGS						
5.5	- medium plasticity, moist to wet at 4.89m BGS						
6.0	END OF BOREHOLE @ 6.10m BGS	6.10					

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

OVERBURDEN LOG 81211.GPJ CRA CORP.GDT 11/6/13



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: MONDELEZ

PROJECT NUMBER: 081211

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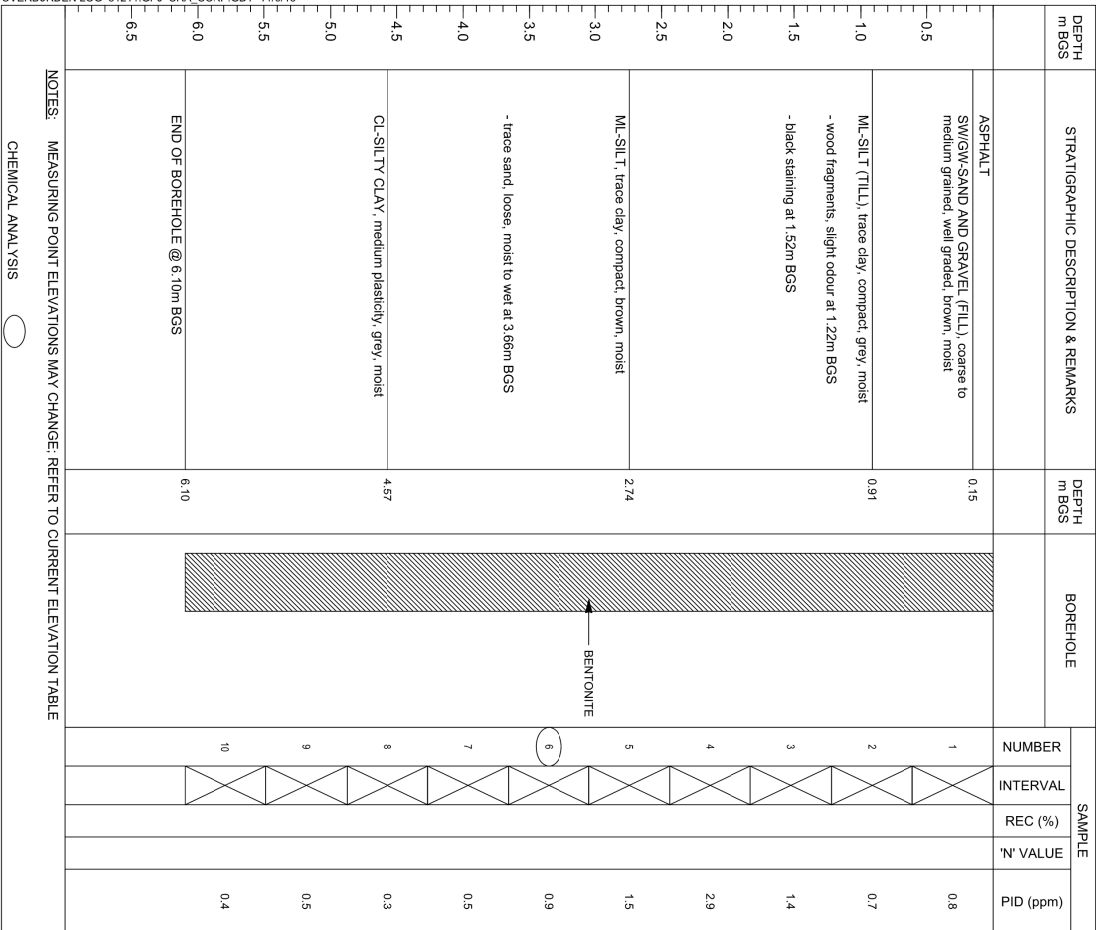
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HOLE DESIGNATION: BH205-13

DATE COMPLETED: 27 May 2013

DILLING METHOD: 51mm O.D./DIRECT PUSH

FIELD PERSONNEL: L. GRIFFITH



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: MONDELEZ

PROJECT NUMBER: 081211

CLIENT:

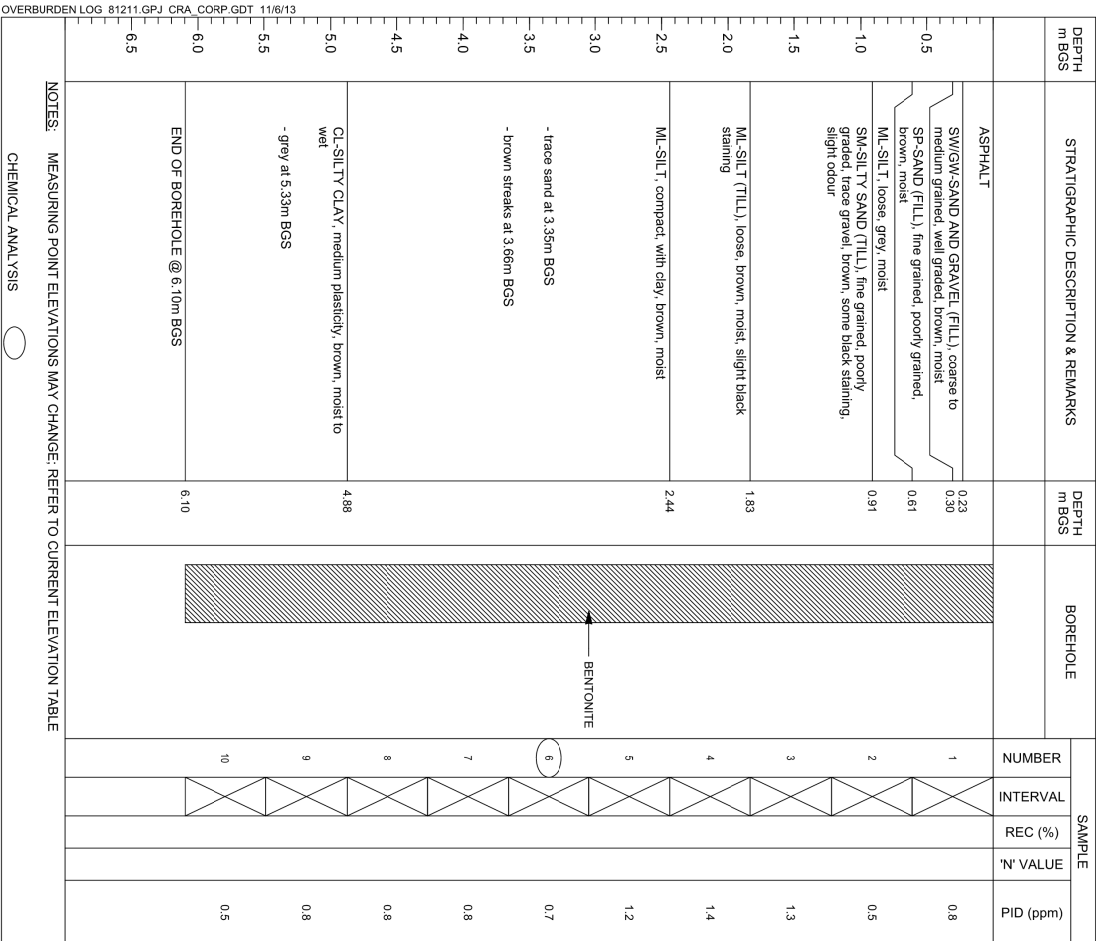
LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH206-13

DATE COMPLETED: 27 May 2013

DILLING METHOD: 51mm O.D./DIRECT PUSH

FIELD PERSONNEL: L. GRIFFITH





STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: MONDELEZ

PROJECT NUMBER: 081211

CLIENT:

LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH207-13

DATE COMPLETED: 27 May 2013

DILLING METHOD: 51mm O.D./DIRECT PUSH

FIELD PERSONNEL: L. GRIFFITH

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (%)	PID (ppm)
0.15	ASPHALT	0.15		1			1.1
0.30	SW/GW SAND AND GRAVEL (FILL), coarse to medium grained, well graded, brown, moist	0.30		2			6.8
0.46	SP-SAND (FILL), medium grained, well graded, brown, moist	0.46		3			50.7
0.61	SM-SILTY SAND, fine grained, poorly graded, moist	0.61		4			33.6
1.0	ML-SILT, trace sand, loose, grey, moist			5			23.6
1.22m BGS	- with sand, black staining at 1.22m BGS			6			1686.0
1.68	SM-SILTY SAND, fine grained, poorly graded, moist, black staining, slight odour	1.68		7			23.6
1.83	ML-SILT, with clay, compact, grey, moist	1.83		8			29.1
2.0				9			21.7
2.5				10			3.6
3.0	- slight odour at 2.44m BGS						
3.5	- some black staining, strong odour at 3.05m BGS						
4.0	- slight odour at 3.66m BGS						
4.5	- moist to wet at 4.27m BGS						
5.0	- slight odour at 4.88m BGS						
5.5	- grey and brown at 5.49m BGS						
6.0	END OF BOREHOLE @ 6.10m BGS	6.10					
6.5							

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (%)	PID (ppm)
0.15	ASPHALT	0.15		1			20.8
0.30	SW/GW SAND AND GRAVEL (FILL), coarse to medium grained, well graded, brown, moist	0.30		2			1.5
0.46	SP-SAND (FILL), medium grained, poorly graded, brown, moist	0.46		3			7.1
1.0	ML-SILT, trace sand, loose, grey, moist			4			4.5
1.22	SM-SILTY SAND, fine grained, poorly graded, grey, some black staining, slight odour	1.22		5			4.8
1.83	ML-SILT, trace sand, loose, grey, moist, slight odour	1.83		6			1.4
3.05	CL-SILTY CLAY, medium plasticity, grey, moist to wet	3.05		7			6.0
3.35m BGS	- slight black staining at 3.35m BGS			8			1.1
4.0	- brown and grey at 3.96m BGS			9			2.9
5.0	- trace sand, slight black staining at 5.18m BGS			10			0.9
6.0	END OF BOREHOLE @ 6.10m BGS	6.10					
6.5							

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: MONDELEZ

PROJECT NUMBER: 081211

CLIENT:

LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH209-13

DATE COMPLETED: 27 May 2013

DILLING METHOD: 51mm O.D./DIRECT PUSH

FIELD PERSONNEL: L. GRIFFITH

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (%)	PID (ppm)
0.09 0.15	ASPHALT			1			1.2
0.61	SW/GW-SAND AND GRAVEL (FILL), coarse to medium grained, well graded, brown, moist SP-SAND (FILL), medium grained, poorly graded, brown, moist, slight oxidation SM-SILTY SAND, fine grained, poorly graded, brown, moist			2			0.7
1.52	ML-SILT, little sand, loose, grey, moist			3			1.9
2.13	- trace sand at 1.98m BGS - trace sand, wood chip fragments, slight odour at 2.13m BGS - slight odour at 2.44m BGS			4			0.7
2.74	- trace sand, medium plasticity, brown, moist at 2.74m BGS			5			1.5
3.66	- with sand, wet at 3.66m BGS			6			0.8
4.27	- trace sand at 4.27m BGS			7			1.3
4.88	- grey, wet at 4.88m BGS			8			0.9
5.18	CL-SILTY CLAY, medium plasticity, grey, moist			9			1.0
6.10	END OF BOREHOLE @ 6.10m BGS			10			0.3

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



STRATIGRAPHIC AND INSTRUMENTATION LOG (OVERBURDEN)

Page 1 of 1

PROJECT NAME: MONDELEZ

PROJECT NUMBER: 081211

CLIENT:

LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH210-13

DATE COMPLETED: 27 May 2013

DILLING METHOD: 51mm O.D./DIRECT PUSH

FIELD PERSONNEL: L. GRIFFITH

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE			
				NUMBER	INTERVAL	REC (%)	PID (ppm)
0.23 0.30	ASPHALT			1			0.5
0.91	SW/GW-SAND AND GRAVEL (FILL), coarse to medium grained, well graded, brown, moist SP-SAND (FILL), medium grained, poorly graded, brown, moist SM-SILTY SAND, fine grained, poorly graded, grey, moist - black staining, slight odour at 1.22m BGS			2			0.8
2.13	ML-SILT, trace clay, fine grained, poorly graded, grey, moist			3			0.9
3.05	- brown at 3.05m BGS - black staining, vegetative rootlets, slight odour at 3.35m BGS - with clay, slight odour at 3.96m BGS			4			1.2
3.96				5			1.0
4.35				6			1.4
4.88				7			3.0
5.18				8			6.2
5.62				9			1.5
6.10	END OF BOREHOLE @ 6.10m BGS			10			0.9

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE; REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS




PROJECT NAME: MONDELEZ
PROJECT NUMBER: 081211

CLIENT: LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH211-13
DATE COMPLETED: 27 May 2013

DRILLING METHOD: 51mm O.D./DIRECT PUSH
FIELD PERSONNEL: L. GRIFFITH

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE				
				NUMBER	INTERVAL	REC (%)	'N' VALUE	PID (ppm)
	ASPHALT	0.15		1	X			0.5
-0.5	SW/GW-SAND AND GRAVEL (FILL), coarse to medium grained, well graded, brown, moist	0.30		2	X			0.5
-1.0	SP-SAND (FILL), medium grained, poorly graded, brown, moist			3	X			0.5
-1.5	SM-SILTY SAND, fine grained, poorly graded, brown, moist	1.22		4	X			0.4
-2.0	ML-SILT, trace clay, fine grained, poorly graded, grey, moist	1.52		5	X			0.2
-3.0	- trace vegetative matter, vegetative staining at 3.05m BGS			6	X			0.4
-4.0				7	X			0.2
-4.5				8	X			0.4
-5.0	- with clay at 4.88m BGS			9	X			0.4
-5.5				10	X			0.6
-6.0	END OF BOREHOLE @ 6.10m BGS	6.10						
-6.5								

NOTES: MEASURING POINT ELEVATIONS MAY CHANGE. REFER TO CURRENT ELEVATION TABLE

CHEMICAL ANALYSIS



PROJECT NAME: MONDELEZ
PROJECT NUMBER: 081211

CLIENT:

LOCATION: 2150 LAKE SHORE BLVD. W., TORONTO, ONTARIO

HOLE DESIGNATION: BH212-13
DATE COMPLETED: 27 May 2013

DRILLING METHOD: 51mm O.D./DIRECT PUSH
FIELD PERSONNEL: L. GRIFFITH

DEPTH m BGS	STRATIGRAPHIC DESCRIPTION & REMARKS	DEPTH m BGS	BOREHOLE	SAMPLE				
				NUMBER	INTERVAL	REC (%)	'N' VALUE	PID (ppm)
0.15	ASPHALT SP-SAND (FILL), medium grained, poorly graded, brown, moist	0.15		1				1.3
0.91	ML-SILT, trace sand - black staining at 1.07m BGS	0.91		2				1.8
	- black staining, slight odour, wood fragments at 1.32m BGS			3				1.2
	- trace clay, slight odour at 2.44m BGS			4				3.4
	- trace clay, grey, moist to wet at 2.74m BGS			5				8.4
	- trace clay, brown, moist at 3.66m BGS			6				1.9
				7				0.8
4.88	END OF BOREHOLE @ 4.88m BGS	4.88		8				0.0